



## APENDICE H

# Estudio de Tránsito

Noviembre 2010

Declaración de Impacto Ambiental – Preliminar

Planta de Generación de Energía Renovable  
y Recuperación de Recursos

BARRIO CAMBALACHE DE ARECIBO

**EnergyAnswers**  
Arecibo

# Traffic Study

## Preliminary Environmental Impact Statement Renewable Power Generation and Resources Recovery Facility

CAMBALACHE - ARECIBO



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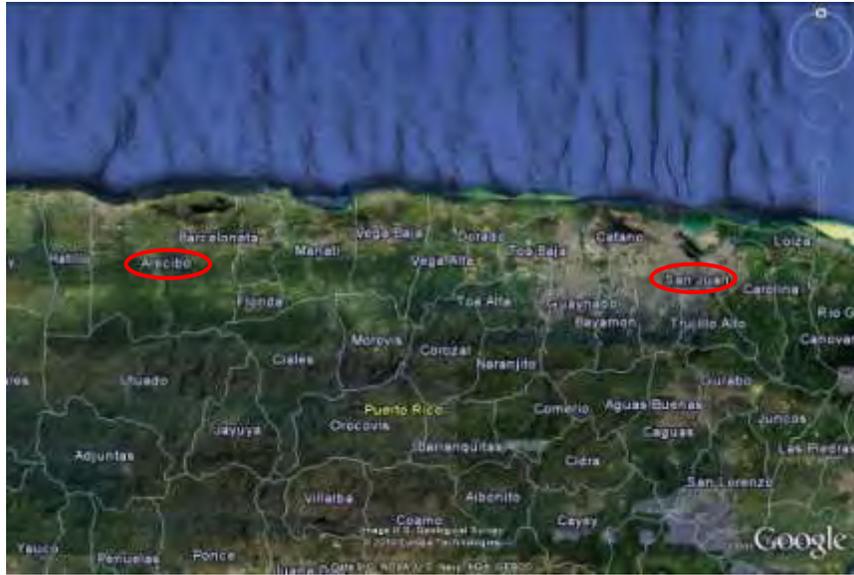
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# 1 EXECUTIVE SUMMARY

Energy Answers International requested CSA Group (CSA) to perform a traffic study to determine the vehicular impact that the construction of the new Renewable Power Generation and Resources Recovery Facility will have to the surrounding area. The survey was prepared and organized according to the parameters set in the Department of Transportation and Public Works (DTPW) official document, "Puerto Rico Guidelines for the Preparation of Traffic and Access Operational Studies". The main purpose for the preparation of this technical document was to analyze and determine the existing and future traffic conditions in the area near the development. The proposed project will be built on the parcel of land where the Arecibo Paper Mill was formerly located. The area to be developed is approximately 82 acres. The current entrance to the estate is located on Highway PR-2, km. 73.1 in the Cambalache Ward of the Municipality of Arecibo.



**Figure 1. Aerial View of Puerto Rico**



**Figure 2. Aerial View of Arecibo in reference to San Juan**



**Figure 3. Project Location**



**Figure 4. Project Location Map**

The vehicular flow generated by the Renewable Power Generation and Resources Recovery Facility as presented in this study, will not adversely affect the vehicular circulation of the area if changes in pavement markings in order to add right turning lanes and traffic signal reprogramming are done according to the study recommendations. During the project construction and as part of the construction of the proposed accesses to the site, a Maintenance of Traffic (MOT) plan shall be deployed and maintained, according to the DTPW guidelines. Once the project is completed, permanent pavement markings and traffic signing shall be installed according to established DTPW guidelines.

## 2 INTRODUCTION

Energy Answers International proposes the construction of the new Renewable Power Generation and Resources Recovery Facility. The project will be located on Highway PR-2, km. 73.1 in the Cambalache Ward of the Municipality of Arecibo. This location houses the remains of the former Arecibo Paper Mill.

The area to be developed is approximately 82 acres. The property is bordered on the north and south by property of the Puerto Rico Land Authority, on the west by the Río Grande de Arecibo, and on the east by Highway PR-2. As part of the proposed development, two accesses to the project are proposed, one exclusively for heavy vehicles and the second one for visitor/employee access.

This traffic study will evaluate the project's potential impact to the principal intersections surrounding the site and determine if improvements are needed, as a result of any possible impact the proposed development might have on said intersections.

### 3 PROJECT DESCRIPTION

The proposed project consists of the construction of a facility where municipal solid waste (MSW) will be used to produce nominal 80 megawatts of electricity and a number of by-products, including Boiler Aggregate™, conditioned fly ash, and recovered ferrous and nonferrous metals. The area to be developed is approximately 82 acres and will include:

- MSW Receiving and Processing Building
- Warehouse
- Processed Refuse Fuel™ (PRF) Storage Building
- Cafeteria, Training, and Lockers Building
- Boiler Building
- Power Block
- Ash Process Building
- Concrete Products Building
- Administration Building

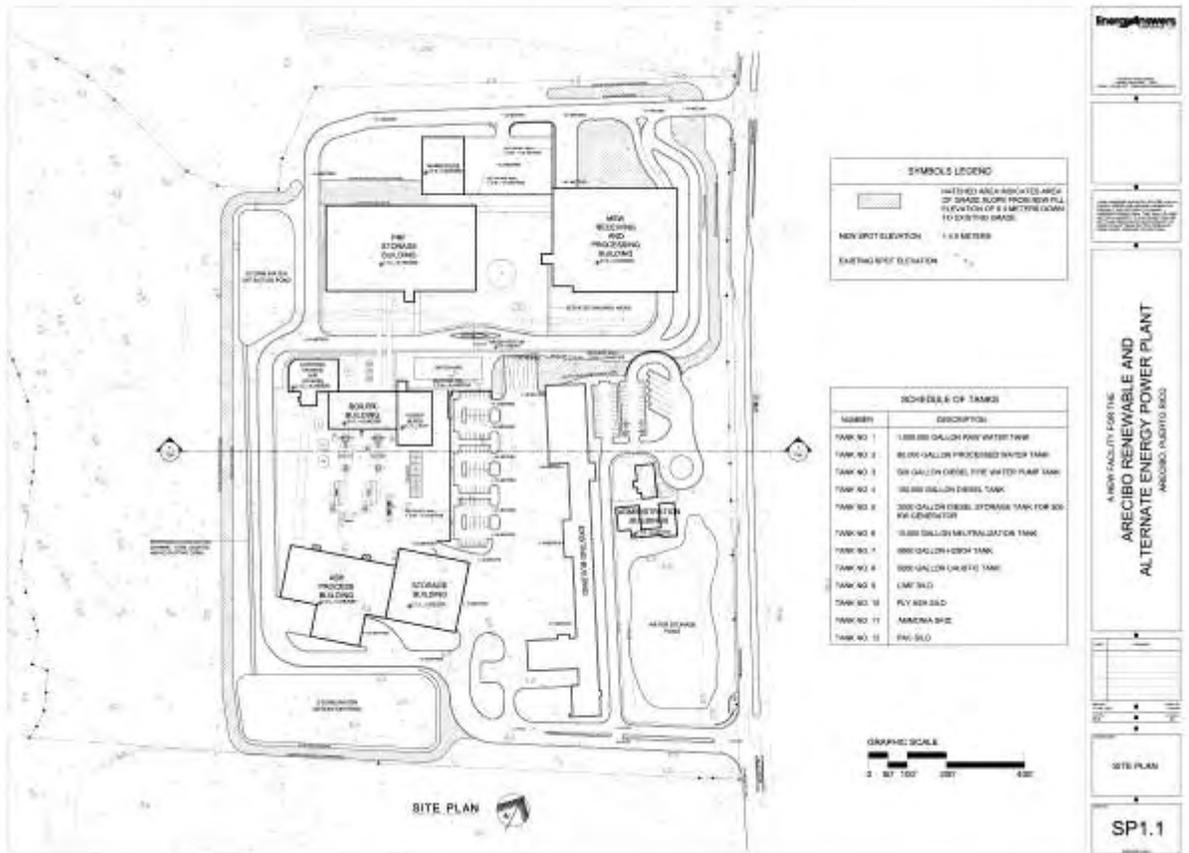


Figure 5. Northwest View of Project



**Figure 6. Northwest Street View of Storage Building**

Two accesses to the project are proposed: a heavy vehicle access and a visitor/employee access. Each access will have two lanes, an entrance and an exit lane. Deceleration and acceleration lanes are proposed for both accesses in order to provide safety for those entering and exiting the project, as well as reduce conflicts with oncoming traffic on Highway PR-2.



**Figure 7. Schematic Site Plan**

## **4 CONDITIONS OF THE ROAD NETWORK**

The road network of the Municipality of Arecibo consists of a toll road, and several primary and secondary main highways of Puerto Rico. Among these are Highways PR-22, PR-2, and PR-10, which provide direct access and connect different state roads to the tertiary road network of the island.

### **4.1 HIGHWAY PR-22**

Highway PR-22 is a toll road which begins in the Municipality of San Juan and extends from east to west to the Municipality of Hatillo. In the vicinity of the proposed development it consists of four lanes separated by a grassy median. Along the entire route, there are six alternating one-way toll plazas. Highway PR-22 connects the municipalities of the northern region of Puerto Rico.

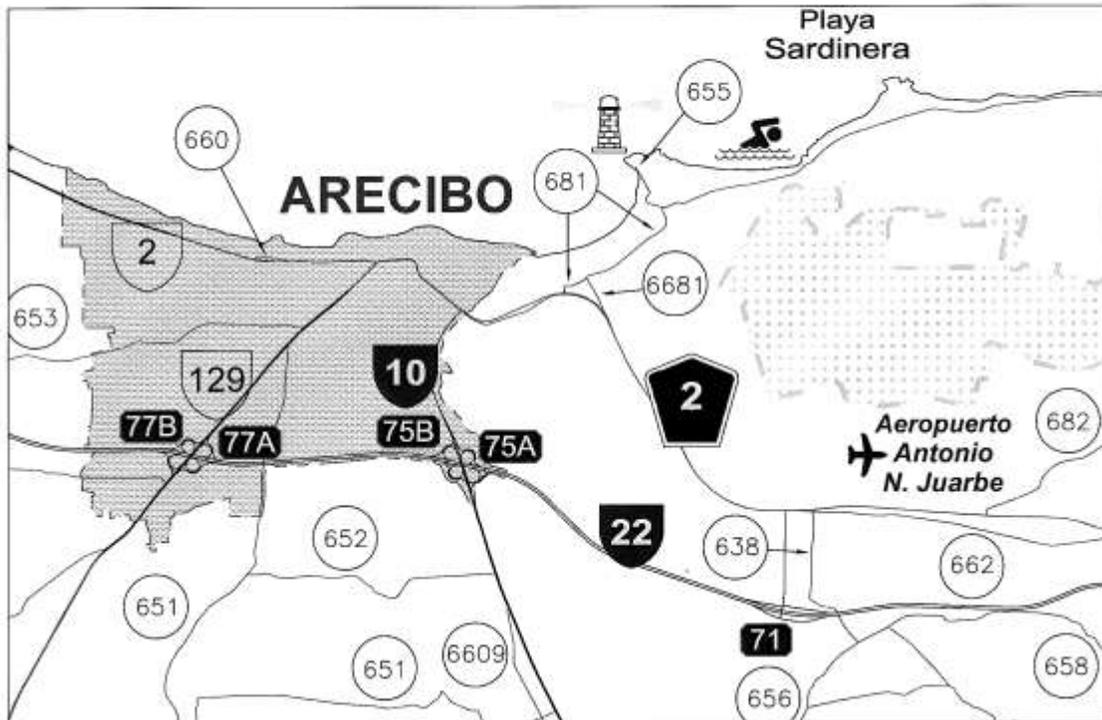
### **4.2 HIGHWAY PR-2**

Highway PR-2 is the longest road in the island's network system. It begins in its intersection with Ponce de León Avenue in the Santurce Sector of the Municipality of San Juan, and extends from east to west connecting all the municipalities of the north region. From the Municipality of Aguadilla, it extends from north to south through the western part of the island, and ends in the Municipality of Ponce.

In the area near the project, Highway PR-2 consists of four lanes divided by a concrete median barrier. There are several left turning lanes at the center of the road, protected by median barriers. In the area of this traffic study, there are two intersections with uncoordinated actuated traffic signals.

### 4.3 HIGHWAY PR-10

Highway PR-10 begins in the Municipality of Ponce and extends from south to north, ending in the Municipality of Arecibo. In the intersection included in this traffic study, it consists of four lanes with no median barrier.



**Figure 8. Arecibo Road Network**

## 5 METHODOLOGY

This study assessed the capacity and the operation of the current and future traffic in the area near the proposed site for the new Renewable Power Generation and Resources Recovery Facility to determine if the proposed development will have an impact on the surrounding road network, and if geometric improvements are needed to mitigate any possible impact on the road system. For this purpose, the following procedures were used:

- Several field inspections were made in order to observe traffic patterns in the area. Also, no highway projects were observed under construction in the surrounding area.
- Proposed project developments in the area were searched in the Access Control Office of the Puerto Rico Highway and Transportation Authority (PRHTA), the Puerto Rico Planning Board (PRPB), and the Puerto Rico Regulations and Permits Administration (PRRPA). From the above mentioned research, the following project was identified; the Oceania residential project, located on Highway PR-681 km. 3.2 in the Islote Ward of Arecibo, was found in the PRPB database. A field inspection confirmed that the project was built and is in full operation. No other proposed projects that have approved construction permits or are under construction were found in the area.

Two proposed projects, located approximately 7 kilometers from the Renewable Power Generation and Resources Recovery Facility, were identified in the PRHTA and the PRPB databases; but, both are still in the permitting process. The Island Cove Hotel and Condo Hotel proposed project will be located in Highway PR-681 km. 7.0 in the Islote Ward of Arecibo. This project is in the process of getting its Environmental Impact Assessment approved. The Corales Apartments proposed project will be located in Highway PR-681 km. 7.6 in the Islote Ward of Arecibo. This project is in the process of celebrating its public hearing.

In addition, a field inspection was performed and no proposed developments were observed under construction in the surrounding area.

- Geometric data and information about the traffic control devices were gathered at the following intersections and segments:
  - PR-2 with PR-10, Llorens Torres Avenue and Juan Rosado Avenue
  - PR-2 with Victor Rojas Avenue

- Linear traffic count at PR-2 km 73.1  
(See Appendix A)
- TeLPEG Engineering, traffic counting specialists, performed traffic data collection on Wednesday, January 27, 2010. Vehicle counting machines collected the data during a 24 hour period. (See Appendix B)
- The morning (AM) and afternoon (PM) peak hours were determined for all intersections being studied. The morning peak hour is from 7:15 AM to 8:15 AM, and the afternoon peak hour is from 2:15 PM to 3:15 PM. (See Appendix C)
- The annual traffic growth rate was analyzed, using the Highway Performance Monitoring System-2008 Classification Log of the Office of Highway Systems of the DTPW. According to this log, the growth rate in 20 years for the intersection of Highways PR-2 and PR-10, and for the intersection of Highway PR-2 and Victor Rojas Avenue, is 54%. The annual traffic growth rate was determined as follows:
  - Growth rate:  $54\% / 20 \text{ years} = 2.7\% \text{ increase/year}$
- The Institute of Transportation Engineers Trip Generation Manual, 7th edition, and the Microtrans Trip Generation Version 5.0 Software were used to estimate the trips generated by the project. Using the expected traffic data provided by Energy Answers, which included the amount and type of truck trips per day and the number of employees, it was determined that the trip generation description which best matched these parameters was the General Light Industrial project description. The AM and PM peak hour entrances and exits were estimated using this data. (See Appendix D)
- Once the traffic data was collected and projected, the capacity of the intersections was analyzed under the current geometric conditions and the effectiveness of the existing traffic control devices was evaluated. The elements of the road network were studied under the following conditions:
  - Existing Condition (2010) - current vehicular volumes were used for the AM and PM peak hours, according to the information gathered by TeLPEG Engineering. Geometric conditions, times and phase movements in the traffic signal systems were assessed. (See Appendix E)
  - Future Condition Opening Year (2013) – the project is presumed to be operating at full capacity by the year 2013. Vehicular volumes were projected to 2013

using the annual traffic growth rate computed above. The AM and PM peak hour flows were used. Geometric conditions, times and phase movements in the traffic signal systems were evaluated. (See Appendix F)

- Horizon Condition 5 years after opening (2018) – Vehicular volumes were projected to 2018 using the annual traffic growth rate computed above. The AM and PM peak hour flows were used. Geometric conditions, times and phase movements in the traffic signal systems were evaluated. (See Appendix G)
- The traffic simulations were done using Trafficware’s Synchro 6 Signal Timing, Capacity Analysis, and Simulation Software. The results were analyzed taking into account the Levels of Service (LOS) and delays at each intersection for each scenario studied. (see Appendix H)
- Conclusions and recommendations based on the results achieved were presented.
- The LOS was used as the main criteria for the assessment of the operating conditions of the road network.

This evaluation criterion includes different types of roads and its various components (ramps, intersections, etc.). The LOS "A" represents excellent and ideal traffic conditions, and LOS "F" represents the worst conditions and vehicular congestion in the facilities. The LOS is based on average delays experienced by vehicles crossing intersections, both signalized and unsignalized.

The categories for each LOS are the following:

*LOS A – Excellent (Free Flow)*

This condition represents free flow accompanied by low volumes of traffic and high speeds. There is little or no restriction in the maneuvers of the driver, who can maintain desired speeds with little or no delay.

*LOS B – Very Good (Reasonably free flow)*

In this condition, operation speeds experience slight restrictions due to the traffic volume. Drivers still have a considerable amount of liberty for choosing their speed and lane used.

*LOS C – Good (Stable flow)*

Speed and maneuvering become more controlled due to higher traffic volumes. The driver's ability to choose their own speed, change lanes, or pass another driver is not always assured. At LOS C most experienced drivers are comfortable, roads remain safely below but efficiently close to capacity, and posted speed is maintained.

*LOS D – Acceptable (Approaching unstable flow)*

This LOS approaches unstable flow, with tolerable operation speeds being maintained, although considerably affected by changes in operating conditions. Drivers experience more restrictions in maneuvering and choosing their own speed.

*LOS E – Capacity (Unstable flow)*

Traffic flow becomes unstable and sudden stops may occur. Vehicular congestion and delays increase considerably. Flow becomes irregular and speed varies rapidly, but rarely reaches the posted limit.

*LOS F – Bad (Forced flow)*

Flow is forced; every vehicle moves in lockstep with the vehicle in front of it, with frequent slowing required.

Table 1 demonstrates the relationship between LOS and average delays per vehicle in signalized and unsignalized intersections. These delays have two components: the delay while crossing the intersection and the delay while stopping at the intersection.

**Table 1. Level of Service (LOS) Criteria for Intersections**

LOS	VEHICLE DELAYS (SECONDS)	
	SIGNALIZED INTERSECTION AND ROUNDABOUTS	UNSIGNALIZED (STOP AND YIELD)
A	$d < 10$	$d < 10$
B	$10 < d < 20$	$10 < d < 15$
C	$20 < d < 35$	$15 < d < 25$
D	$35 < d < 55$	$25 < d < 35$
E	$55 < d < 80$	$35 < d < 50$
F	$80 < d$	$50 < d$

The processes used to complete this study are in accordance with the guidelines established by the PRHTA of the DTPW, the design standards of the American Association of State Highway and Transportation Officials (AASHTO), the Transportation Research Board (TRB), and the Institute of Transportation Engineers (ITE) manuals. To complete this traffic study, Trafficware's Synchro 6 Signal Timing, Capacity Analysis, and Simulation Software was used, as well as the TRB's Highway Capacity Manual 2000 (HCM2000) and the ITE traffic assessment guidelines.

## 6 TRIP GENERATION AND DISTRIBUTION

To determine the trips generated by the new Renewable Power Generation and Resources Recovery Facility, the Trip Generation Manual of the ITE (7th edition) and Microtrans Trip Generation Version 5.0 software were used. Energy Answers provided the expected traffic data, which included the amount and type of truck trips per day and the number of employees (see Appendix I). This traffic data represents the impact throughout one weekday. For the traffic study purpose, the morning and afternoon peak hours were evaluated, since they are the most critical.

The trip generation description which best matched the conditions of the project was the General Light Industrial project description. Table 2 shows the estimated trips generated by the project during the AM and PM peak hours, using the mentioned parameters and the Trip Generation Software.

**Table 2. Trips Generated by the Resource Recovery Facility**

Peak Hour Volume (AM)		Peak Hour Volume (PM)	
Enter	Exit	Enter	Exit
56	11	14	50

According to the data provided by Energy Answers, it was determined that 30% of the trips generated are cars and 70% of the trips generated are heavy vehicles. Table 3 shows the vehicle type distribution for the trips generated during the AM and PM peak hours.

**Table 3. Vehicle Type Distribution**

Vehicle Type	Peak Hour Volume (AM)		Peak Hour Volume (PM)	
	Enter	Exit	Enter	Exit
Cars	17	3	4	15
Heavy vehicles	39	8	10	35
<b>Total</b>	56	11	14	50

Access #1, the north entrance, will be used for heavy vehicles only. Energy Answers estimates that 75% of the heavy vehicles will travel from the eastern part of the island, and 25% will travel from the west.

Access #2, the south entrance, will be mainly used as the employee/visitor entrance. For purposes of this traffic study, it was assumed that 50% of the cars will travel from the eastern part of the island, and 50% will travel from the west.

Tables 4 through 11 show the trip generation distributions for the AM and PM peak hours at each intersection.

**Table 4. Intersection #1: PR-2, PR-10 and Juan Rosado Avenue**

**AM Peak Hour Trip Generation Distribution per Movement**

	Movement	Cars	Heavy vehicles	Total
PR-2	EB-L			0
	EB-T			0
	EB-R			0
	WB-L	1	2	3
	WB-T			0
	WB-R			0
PR-10	NB-L			0
	NB-T			0
	NB-R	8	10	18
Juan	SB-L			0

	Movement	Cars	Heavy vehicles	Total
Rosado Avenue	SB-T			0
	SB-R			0

**Table 5. Intersection #2: PR-2 and Victor Rojas Avenue**  
AM Peak Hour Trip Generation Distribution per Movement

	Movement	Cars	Heavy vehicles	Total
PR-2	EB-L			0
	EB-T	8	10	18
	WB-T	1	2	3
	WB-R			0
Victor Rojas Avenue	NB-T			0
	NB-R			0

**Table 6. Intersection #3: PR-2 and Access #1**  
AM Peak Hour Trip Generation Distribution per Movement

	Movement	Cars	Heavy vehicles	Total
Access #1	EB-L		2	2
	EB-R		6	6
PR-2	NB-L		29	29
	NB-T	1		1
	SB-T	8		8
	SB-R		10	10

**Table 7. Intersection #4: PR-2 and Access #2**

**AM Peak Hour Trip Generation Distribution per Movement**

	<b>Movement</b>	<b>Cars</b>	<b>Heavy vehicles</b>	<b>Total</b>
Access #2	EB-L	1		1
	EB-R	2		2
PR-2	NB-L	9		9
	NB-T		29	29
	SB-T		6	6
	SB-R	8		8

**Table 8. Intersection #1: PR-2, PR-10 and Juan Rosado Avenue**

**PM Peak Hour Trip Generation Distribution per Movement**

	<b>Movement</b>	<b>Cars</b>	<b>Heavy vehicles</b>	<b>Total</b>
PR-2	EB-L			0
	EB-T			0
	EB-R			0
	WB-L	7	9	16
	WB-T			0
	WB-R			0
PR-10	NB-L			0
	NB-T			0
	NB-R	2	2	4
Juan Rosado Avenue	SB-L			0
	SB-T			0
	SB-R			0

**Table 9. Intersection #2: PR-2 and Victor Rojas Avenue**

**PM Peak Hour Trip Generation Distribution per Movement**

	Movement	Cars	Heavy vehicles	Total
PR-2	EB-L			0
	EB-T	2	2	4
	WB-T	7	9	16
	WB-R			0
Victor Rojas Avenue	NB-T			0
	NB-R			0

**Table 10. Intersection #3: PR-2 and Access #1**

**PM Peak Hour Trip Generation Distribution per Movement**

	Movement	Cars	Heavy vehicles	Total
Access #1	EB-L		9	9
	EB-R		26	26
PR-2	NB-L		8	8
	NB-T	7		7
	SB-T	2		2
	SB-R		2	2

**Table 11. Intersection #4: PR-2 and Access #2**  
**PM Peak Hour Trip Generation Distribution per Movement**

	<b>Movement</b>	<b>Cars</b>	<b>Heavy vehicles</b>	<b>Total</b>
Access #2	EB-L	7		7
	EB-R	8		8
PR-2	NB-L	2		2
	NB-T		8	8
	SB-T		26	26
	SB-R	2		2

## 7 OPERATIONAL ANALYSIS

The capacity analysis was completed using the HCM2000 guidelines. The traffic simulations were done using Trafficware’s Synchro 6 Signal Timing, Capacity Analysis, and Simulation Software. Both signalized intersections (PR-2/PR-10/Juan Rosado Avenue and PR-2/Victor Rojas Avenue) were analyzed and optimized. The current and proposed geometric conditions of Highway PR-2 in front of the project were evaluated.

The area was studied under current conditions (2010), for future conditions in 2013 (the project is presumed to be operating at full capacity by this year), and for the horizon conditions in 2018, 5 years after opening day. Existing geometric conditions were evaluated for the AM and PM peak hours using the existing vehicular flows to establish the current LOS and delays. The current volumes were projected through 2013 and 2018. Using these volumes with the data gathered from the Trip Generation Software, the intersections were evaluated, compared and optimized (if necessary) in order to achieve or maintain adequate LOS and delays, according to the following guidelines established in the “Puerto Rico Guidelines for the Preparation of Traffic Access Operational Studies” of the DTPW, and shown in Tables 12 and 13.

**Table 12. Average delay impact in a signalized intersection**

<b>Existing LOS</b>	<b>Average additional delay allowed with project</b>
A	20 sec/veh
B	20 sec/veh
C	15 sec/veh
D	15 sec/veh
E	Average delay shall not be $\geq$ 80 sec/veh
F	Automatically provide alternatives

**Table 13. Average delay impact in an unsignalized intersection**

<b>Existing LOS</b>	<b>Average additional delay allowed with project</b>
A	15 sec/veh
B	15 sec/veh
C	10 sec/veh
D	10 sec/veh
E	Average delay shall not be $\geq$ 50 sec/veh
F	Automatically provide alternatives

In order not to exceed these allowed additional delays, exclusive right turning lanes were added for the northbound movement at Intersection #1 (PR-2, PR-10, and Juan Rosado Avenue), and for the westbound movement at Intersection #2 (PR-2 and Victor Rojas Avenue). At Intersection #1, the center lane, currently being used exclusively for the through eastbound movement, will be a shared lane for the left turn and through movements. Also at this intersection, the lane widths for the southbound movements were changed to accommodate an additional third lane. Traffic signal times at Intersections #1 and #2 were optimized.

Access #1, a heavy vehicle entrance, was first analyzed in the year 2013 as an unsignalized intersection. The LOS for the intersection was A (Excellent); the LOS for the northbound left turning movement was B (Very Good) and C (Good) for the AM and PM peak hours, respectively. For the vehicles exiting the premises (eastbound movement), delays of 124.2 seconds and 229.6 seconds were observed for both peak hours. However, the maximum queue for this access was two vehicles. This queue does not affect the operations of the proposed project or, more importantly, the traffic flow in Highway PR-2.

As per Energy Answers' request, CSA included as part of the improvements proposed for the project a traffic signal at Access #1, for ease of movement and safety precautions. After analyzing the intersection under this condition, a LOS of C (Good) or better was observed for all movements.

The results for the AM and PM peak hours at each intersection are shown in Tables 14 and 15.

**Table 14. Intersection Results: AM Peak Hour**

Intersection	LOS / Delay (sec)						
	Existing (2010)	2013	Difference (sec)	2013 with improvements	Difference (sec)	2018 with improvements	Difference (sec)
PR-2, PR-10, & Juan Rosado Ave.	E/79.0	F/108.8	-29.80	D/51.6	27.4	D/53.9	25.1
PR-2 & Victor Rojas Ave.	F/185.1	F/230.5	-45.40	B/16.0	169.1	D/36.6	148.5
PR-2 & Access #1	N/A	A/0.4	N/A	B/10.7	N/A	C/28.5	N/A
PR-2 & Access #2	N/A	A/0.0	N/A	A/0.0	N/A	A/0.0	N/A

**Table 15. Intersection Results: PM Peak Hour**

Intersection	LOS / Delay (sec)						
	Existing (2010)	2013	Difference (sec)	2013 with improvements	Difference (sec)	2018 with improvements	Difference (sec)
PR-2, PR-10, & Juan Rosado Ave.	D/45.9	E/58.2	-12.3	D/42.7	3.2	D/50.0	-4.1
PR-2 & Victor Rojas Ave.	E/61.6	F/97.6	-36.0	C/20.9	40.7	C/27.4	34.2
PR-2 & Access #1	N/A	A/2.9	N/A	A/4.3	N/A	A/5.2	N/A
PR-2 & Access #2	N/A	A/0.1	N/A	A/0.1	N/A	A/0.2	N/A

## 8 SUMMARY OF RESULTS

The results for the AM and PM peak hours for each movement are presented in Appendix J. The analysis was completed using SYNCHRO 6 for the existing and future conditions with the trips generated by the project.

### **Intersection #1: PR-2, PR-10, and Juan Rosado Avenue**

At present, drivers are using the PR-10 shoulder as an exclusive right turning lane. In order to comply with the allowed delays for the northbound right turning movement, an exclusive right turning lane was added in PR-10 during the analysis. Other improvements proposed in this intersection are to change the current use for the center eastbound lane of PR-2, from being an exclusive through movement, to a shared lane for the left turn and through movements. Also, the lane widths for Juan Rosado Avenue have to be reduced in order to accommodate an additional third lane for the right turning southbound movement.

The traffic signal times should be changed for the AM and PM hours, as shown in Table 16.

**Table 16. Changes in traffic signal times at Intersection #1**

<b>Movement</b>	<b>Existing Times (Green+Yellow+All Red), in seconds</b>	<b>Proposed Times (Green+Yellow+All Red) for AM hours, in seconds</b>	<b>Proposed Times (Green+Yellow+All Red) for PM hours, in seconds</b>
EB-L	24.5	14.0	29.0
EB-T	59.5	31.0	63.5
WB-L	24.5	36.0	39.0
WB-T	59.5	53.0	73.5
NB	44.5	37.0	51.0
SB	24.5	16.0	23.5

### **Intersection #2: PR-2 and Victor Rojas Avenue**

At present, drivers are using the PR-2 westbound shoulder as an exclusive right turning lane. In order to comply with the allowed delays for this movement, an exclusive right turning lane was added in PR-2 during the analysis.

The traffic signal time for the eastbound left turning movement should be changed for the PM hours, as shown in Table 17.

**Table 17. Changes in traffic signal times at Intersection #2**

<b>Movement</b>	<b>Existing Times (Green+Yellow+All Red), in seconds</b>	<b>Proposed Times (Green+Yellow+All Red) for AM hours, in seconds</b>	<b>Proposed Times (Green+Yellow+All Red) for PM hours, in seconds</b>
EB-L	24.0	24.0	26.0
EB-T	64.0	64.0	64.0
WB	40.0	40.0	40.0
SB	55.0	55.0	55.0

**Intersection #3: PR-2 and Access #1**

A traffic signal is proposed for this intersection. Table 18 shows the proposed times for the AM and PM hours.

**Table 18. Traffic signal times proposed at Intersection #3**

<b>Movement</b>	<b>Proposed Times (Green+Yellow+All Red) for AM and PM hours, in seconds</b>
EB	25.0
NB-L	20.0
NB-T	45.0
SB	25.0

**Intersection #4: PR-2 and Access #2**

Highway PR-2 runs freely at this intersection, except for the northbound left movement. Delays for this movement are not significant.

## 9 CONCLUSIONS AND RECOMMENDATIONS

This Traffic Study completed in the road network serving the new Renewable Power Generation and Resources Recovery Facility project has concluded that the vehicular flow generated by this project will not adversely affect traffic operations in the area. However, the following recommendations should be considered:

### **Intersection #1: PR-2, PR-10, and Juan Rosado Avenue**

At present, drivers are using the PR-10 shoulder as an exclusive right turning lane. It is recommended that the shoulder pavement marking be erased and a right-only lane with a storage length of 18.3 m (60 ft) be marked. Traffic signs indicating this is a right-only lane should also be installed.

Another improvement proposed at this intersection is to change the current use for the center eastbound lane of PR-2, from being an exclusive through movement, to a shared lane for the left turn and through movements. Traffic signs and pavement marking symbols indicating the new lane uses should be installed.

At Juan Rosado Avenue, the lane widths should be reduced to 10 ft each in order to accommodate an additional third lane for the right turning southbound movement. Existing pavement markings should be erased and new markings should be painted at the final widths. The right turning lane should have a storage length of 18.3 m (60 ft). Traffic signs and pavement marking symbols indicating the new lane uses should be installed.

The traffic signal times should also be changed as previously discussed.

### **Intersection #2: PR-2 and Victor Rojas Avenue**

At present, drivers are using the PR-2 westbound shoulder as an exclusive right turning lane. It is recommended that the shoulder pavement marking be erased and a right-only lane with a

storage length of 122 m (400 ft) be marked. Traffic signs and pavement marking symbols indicating the new lane uses should be installed

The traffic signal times should also be changed as previously discussed.

### **Intersection #3: PR-2 and Access #1**

The project proposes a 122 m (400 ft) deceleration lane and a 107 m (350 ft) acceleration lane to enter and exit the project for southbound traffic. A 107 m (350 ft) left turning lane is proposed for northbound entering traffic. Proper pavement markings and traffic signing should be installed.

A traffic signal system will be installed at this intersection. Signal times were indicated in the Summary of Results section of this study.

### **Intersection #4: PR-2 and Access #2**

The project proposes a 122 m (400 ft) deceleration lane and a 107 m (350 ft) acceleration lane to enter and exit the project for southbound traffic. A 107 m (350 ft) left turning lane is proposed for northbound entering traffic. Proper pavement markings and traffic signing should be installed. Drivers should be alerted with traffic signs that a truck crossing will be encountered ahead.

During the project construction, a Maintenance of Traffic (MOT) plan shall be performed, according to the DTPW guidelines. Once the project is completed, pavement markings and traffic signing shall be placed according to the MUTCD 2009 Edition.

## 10 REFERENCES

- Highway Capacity Manual 2000. Transportation Research Board, Washington, D.C., 2000.
- A Policy on Geometric Design of Highways and Streets. American Association of State Highway and Transportation Officials, Washington, D.C., 2001.
- Highway Design Manual. Departamento de Transportación y Obras Públicas, San Juan, PR, 1979.
- Transportation and Land Development. Institute of Transportation Engineering, Washington, D.C., 2002.
- <http://www.dtop.gov.pr>
- <http://www.jp.gobierno.pr>
- <http://www.arpe.org>
- Guías para la Preparación de Estudios Operacionales, Departamento de Transportación y Obras Públicas, San Juan, PR, 22 de diciembre de 2004
- Google Earth

## 11 CERTIFICATION

I, Ruth M. Vargas Vidal, married and resident of Trujillo Alto, Puerto Rico hereby certify that I am a professional member in good standing of the Professional Association of Engineers and Land Surveyors of Puerto Rico, with license PE 16565. I also certify that I have prepared and thoroughly reviewed the Renewable Power Generation and Resources Recovery Facility Traffic Study requested by Energy Answers International. All the information presented in this document is valid to this date.



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Ruth M. Vargas Vidal

PE 16565

September 10, 2010

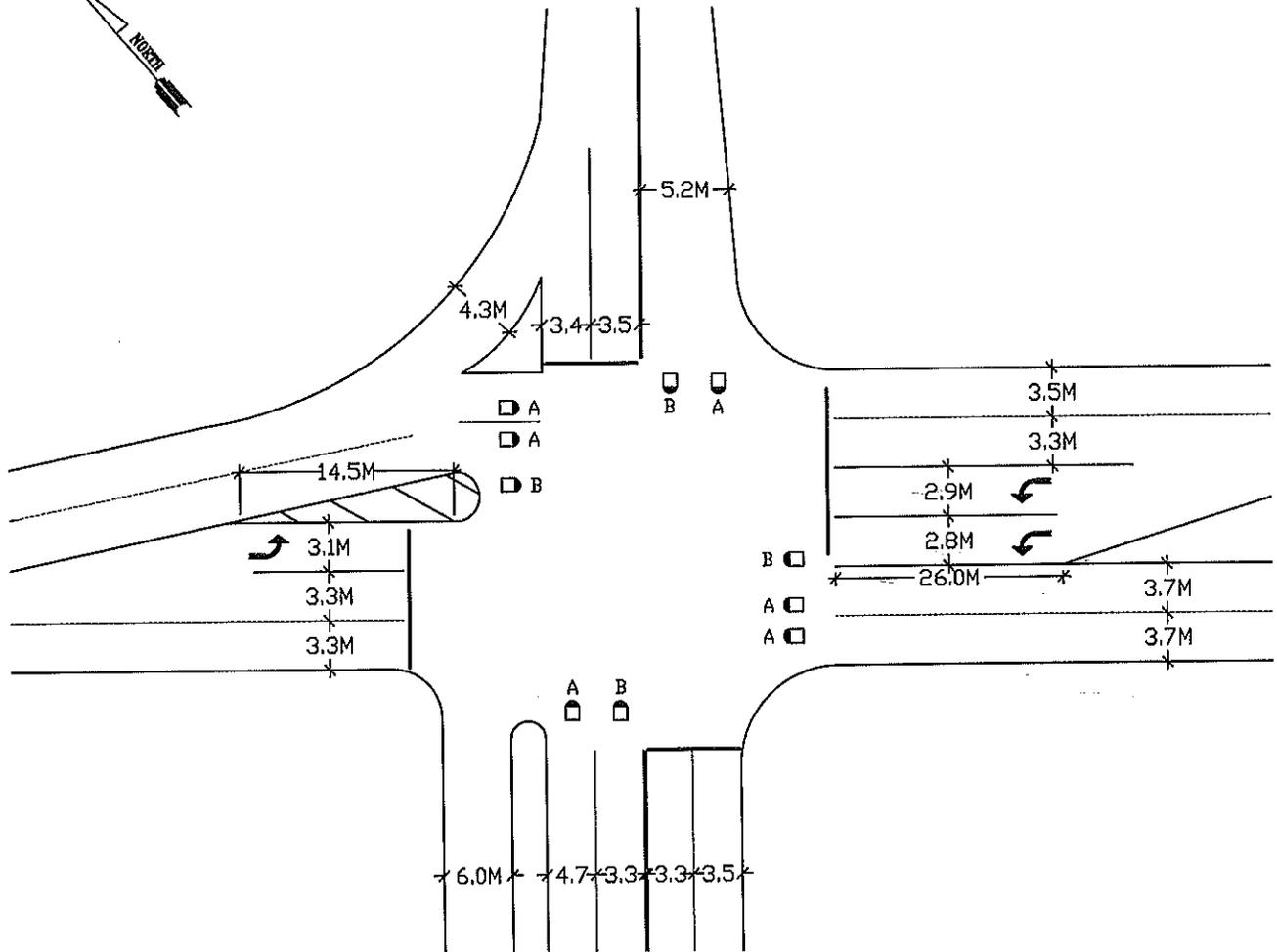
## **APPENDICES**



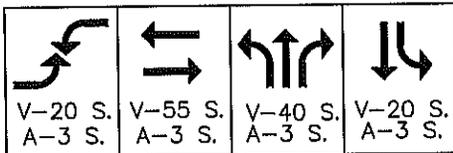
# APPENDIX – A

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# PR-2 & PR-10 AND JUAN ROSADO AVENUE

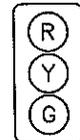


PHASE 1 PHASE 2 PHASE 3 PHASE 4

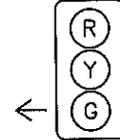


ALL RED = 1.5 S.

SIGNAL HEAD  
MKD-A



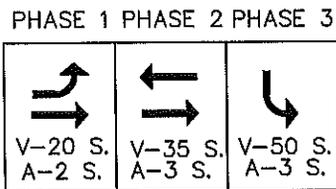
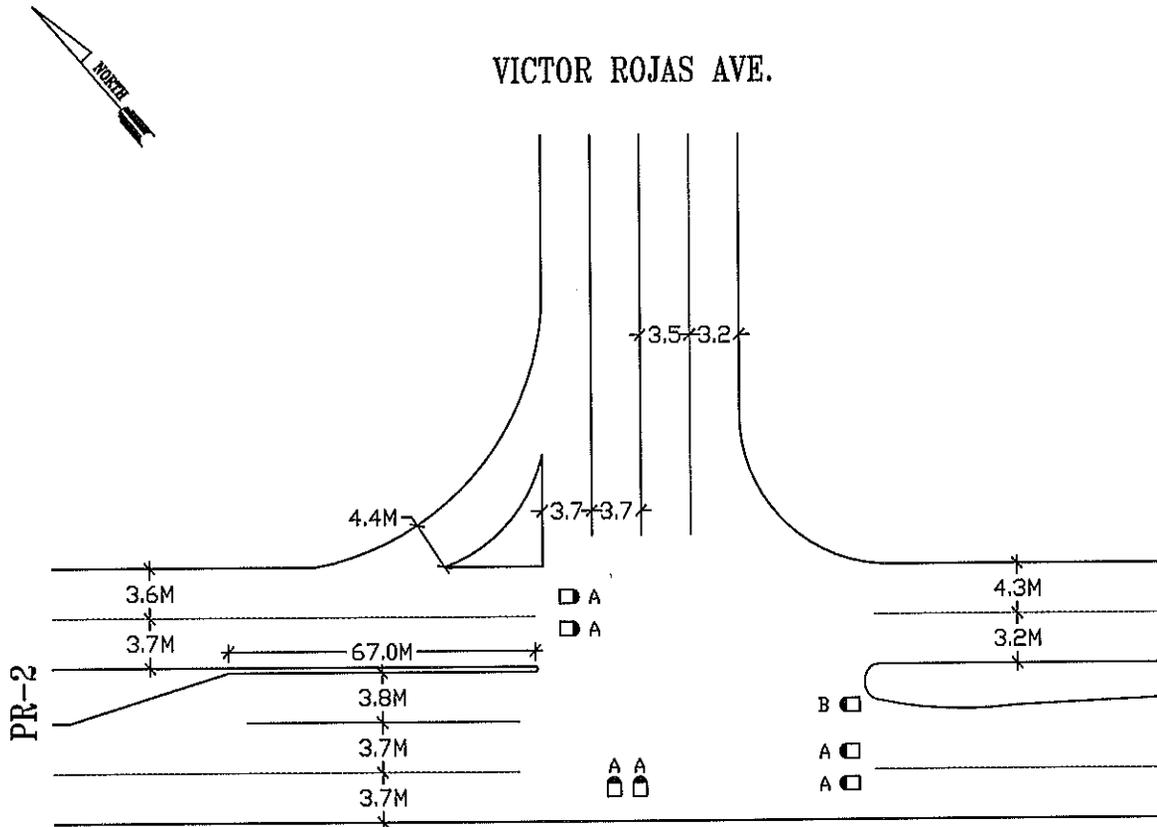
SIGNAL HEAD  
MKD-B



**TELPEG ENGINEERING**  
TRAFFIC COUNTING SPECIALISTS

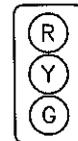
O: 787-366-1352 E: TELPEG@GMAIL.COM

# PR-2 & VICTOR ROJAS AVENUE, ARECIBO, P.R.

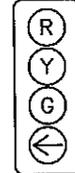


ALL RED = 2 S.

SIGNAL HEAD  
MKD-A



SIGNAL HEAD  
MKD-B



**TELPEG ENGINEERING**  
TRAFFIC COUNTING SPECIALISTS



# APPENDIX – B

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**INTERSECTION: PR-2 & PR-10 AND JUAN ROSADO AVE., ARECIBO, P.R.**  
**DATE: WEDNESDAY, JANUARY 27, 2010**

TIME PERIOD	PR-10			J. ROSADO AVE.			PR-2						TOTAL 15 MIN	TOTAL 60 MIN
	NB-L	NB-T	NB-R	SB-L	SB-T	SB-R	EB-L	EB-T	EB-R	WB-L	WB-T	WB-R		
12:00 AM	4	1	10	3	3	2	2	12	8	6	10	2	63	
12:15 AM	2	0	11	3	1	1	0	13	0	4	7	1	43	
12:30 AM	6	2	3	1	2	2	2	4	8	3	5	1	39	
12:45 AM	0	0	10	3	1	0	1	12	3	5	7	1	43	188
1:00 AM	1	0	8	2	2	1	0	10	2	3	5	1	35	160
1:15 AM	5	1	4	1	2	1	1	5	2	2	3	1	28	145
1:30 AM	2	1	6	2	2	2	1	8	5	1	2	0	32	138
1:45 AM	1	0	4	1	1	1	1	4	3	2	2	0	20	115
2:00 AM	2	0	6	2	1	0	1	8	3	2	3	1	29	109
2:15 AM	2	1	2	0	2	2	2	2	6	2	4	1	26	107
2:30 AM	2	0	5	1	2	2	2	6	10	3	5	1	39	114
2:45 AM	2	1	2	1	1	0	2	3	9	3	4	1	29	123
3:00 AM	3	1	4	1	1	1	2	5	7	2	4	1	32	126
3:15 AM	1	0	4	1	1	0	5	5	18	3	4	1	43	143
3:30 AM	2	0	5	1	1	1	2	6	6	4	5	1	34	138
3:45 AM	11	4	4	1	2	1	4	5	14	4	5	1	56	165
4:00 AM	7	2	6	2	1	1	8	8	32	6	10	2	85	218
4:15 AM	4	1	13	4	1	1	4	16	18	7	10	2	81	256
4:30 AM	11	4	6	2	3	2	8	7	34	10	14	2	103	325
4:45 AM	14	5	6	2	2	1	6	7	24	16	24	4	111	380
5:00 AM	8	2	16	4	1	0	3	18	13	18	27	4	114	409
5:15 AM	11	3	16	4	3	2	6	19	22	20	31	5	142	470
5:30 AM	11	3	18	5	4	3	7	21	26	30	45	7	180	547
5:45 AM	24	8	12	3	4	4	9	14	34	28	43	7	190	626
6:00 AM	34	11	14	4	3	2	8	16	34	36	55	9	226	738
6:15 AM	52	17	16	4	4	4	6	19	25	60	89	15	311	907
6:30 AM	68	22	22	6	8	6	10	26	38	95	143	24	468	1195
6:45 AM	82	27	20	6	21	18	15	24	62	93	139	23	530	1535
7:00 AM	92	30	28	8	13	10	15	33	62	133	199	33	656	1965
7:15 AM	104	34	35	10	19	16	12	41	47	156	233	39	746	2400
7:30 AM	124	41	37	10	23	18	12	43	46	124	185	31	694	2626
7:45 AM	105	35	39	11	31	26	14	46	56	118	177	29	687	2783
8:00 AM	84	28	47	13	21	18	13	55	54	130	194	32	689	2816
8:15 AM	83	28	47	13	18	15	23	55	90	106	158	26	662	2732
8:30 AM	90	30	38	11	28	22	26	45	105	105	158	26	684	2722
8:45 AM	103	34	48	13	33	27	24	57	95	87	131	22	674	2709
9:00 AM	80	27	41	11	26	22	25	48	102	91	136	23	632	2652
9:15 AM	95	32	53	15	31	25	22	62	90	71	107	18	621	2611
9:30 AM	75	25	60	17	28	23	22	71	89	83	124	21	638	2565
9:45 AM	83	27	60	17	30	24	25	71	101	71	106	18	633	2524
10:00 AM	77	25	59	16	28	23	23	70	92	83	124	21	641	2533
10:15 AM	80	26	66	18	26	22	23	78	91	77	115	19	641	2553
10:30 AM	74	24	62	17	24	19	23	72	93	71	107	18	604	2519
10:45 AM	57	19	78	21	25	20	25	92	99	68	101	17	622	2508
11:00 AM	65	21	68	19	32	26	23	80	90	76	113	19	632	2499
11:15 AM	69	23	70	19	31	26	21	82	85	82	122	20	650	2508
11:30 AM	81	27	66	18	28	23	27	77	106	74	110	18	655	2559
11:45 AM	76	25	67	18	40	32	27	79	107	70	104	17	662	2599
12:00 PM	71	24	74	20	27	22	22	87	88	78	118	19	650	2617
12:15 PM	62	21	70	19	26	21	22	82	86	83	124	20	636	2603
12:30 PM	77	25	78	21	31	25	21	92	84	84	126	21	685	2633

Peak Hour

TIME PERIOD	PR-10			J. ROSADO AVE.			PR-2						TOTAL	TOTAL
	NB-L	NB-T	NB-R	SB-L	SB-T	SB-R	EB-L	EB-T	EB-R	WB-L	WB-T	WB-R	15 MIN	60 MIN
12:45 PM	77	26	75	21	30	25	22	88	87	91	136	22	700	2671
1:00 PM	68	22	74	20	20	16	23	87	92	78	117	19	636	2657
1:15 PM	65	21	74	20	26	22	22	86	87	83	125	21	652	2673
1:30 PM	71	23	68	19	36	29	26	80	106	80	121	20	679	2667
1:45 PM	84	28	68	19	30	24	25	80	101	81	121	20	681	2648
2:00 PM	77	25	72	20	29	23	24	85	94	90	134	22	695	2707
<b>2:15 PM</b>	86	28	73	20	32	27	22	86	86	90	134	22	706	2761
<b>2:30 PM</b>	65	21	82	23	37	30	23	97	90	86	129	21	704	2786
<b>2:45 PM</b>	52	17	72	20	29	24	27	85	108	96	144	24	698	2803
<b>3:00 PM</b>	59	20	83	23	27	22	26	98	104	96	145	24	727	<b>2835</b>
3:15 PM	86	29	75	21	31	25	24	88	94	84	125	21	703	2832
3:30 PM	53	18	79	22	26	22	21	93	84	76	113	19	626	2754
3:45 PM	59	20	76	21	36	29	22	89	87	74	112	18	643	2699
4:00 PM	33	11	94	26	31	25	20	111	81	68	101	17	618	2590
4:15 PM	56	18	82	22	32	26	24	96	97	74	110	18	655	2542
4:30 PM	39	13	78	22	32	26	23	92	93	75	112	18	623	2539
4:45 PM	54	18	78	21	39	32	31	91	123	70	105	17	679	2575
5:00 PM	50	16	80	22	23	19	18	94	74	61	92	15	564	2521
5:15 PM	29	9	89	25	14	12	12	105	49	61	91	15	511	2377
5:30 PM	34	11	78	22	11	9	16	92	65	60	91	15	504	2258
5:45 PM	32	10	70	19	17	13	13	82	54	42	64	11	427	2006
6:00 PM	27	9	82	23	10	8	6	96	26	50	76	13	426	1868
6:15 PM	32	10	65	18	12	9	12	76	46	43	64	11	398	1755
6:30 PM	31	10	53	15	12	9	13	63	53	42	64	10	375	1626
6:45 PM	35	11	60	17	6	4	11	71	42	42	64	10	373	1572
7:00 PM	35	12	49	13	9	8	17	57	68	39	58	10	375	1521
7:15 PM	44	15	47	13	12	10	12	55	47	35	53	9	352	1475
7:30 PM	30	10	47	13	7	6	4	55	18	31	46	8	275	1375
7:45 PM	28	9	39	11	10	9	13	46	51	26	40	7	289	1291
8:00 PM	17	6	51	14	8	6	6	60	26	26	40	7	267	1183
8:15 PM	24	8	41	11	8	7	12	48	50	23	35	6	273	1104
8:30 PM	17	5	37	10	8	7	9	44	35	18	27	4	221	1050
8:45 PM	15	5	41	11	15	13	8	48	32	20	29	5	242	1003
9:00 PM	13	4	38	11	9	7	8	45	31	24	37	6	233	969
9:15 PM	11	3	34	9	9	7	5	40	22	21	32	5	198	894
9:30 PM	6	2	30	8	6	4	6	36	26	22	34	6	186	859
9:45 PM	8	2	28	8	7	6	12	32	47	18	27	4	199	816
10:00 PM	6	2	26	7	4	4	8	31	33	16	25	4	166	749
10:15 PM	5	1	29	8	6	4	3	34	11	21	32	5	159	710
10:30 PM	6	2	24	7	4	4	5	28	22	17	26	4	149	673
10:45 PM	9	3	29	8	2	2	3	34	10	9	14	2	125	599
11:00 PM	8	3	20	6	4	3	2	24	6	12	17	3	108	541
11:15 PM	2	0	22	6	4	4	0	26	1	14	21	4	104	486
11:30 PM	7	2	16	5	5	4	1	19	2	10	15	2	88	425
11:45 PM	4	1	17	5	2	2	1	20	3	10	14	2	81	381
<b>TOTAL</b>	<b>3928</b>	<b>1287</b>	<b>4089</b>	<b>1130</b>	<b>1466</b>	<b>1193</b>	<b>1249</b>	<b>4814</b>	<b>4988</b>	<b>4794</b>	<b>7189</b>	<b>1192</b>		

Peak Hour

**TELPEG ENGINEERING**  
TRAFFIC COUNTING SPECIALISTS

O: 787-366-1352 E: TELPEG@GMAIL.COM

**INTERSECTION: PR-2 & VICTOR ROJAS AVENUE, ARECIBO, P.R.**  
**DATE: WEDNESDAY, JANUARY 27, 2010**

TIME PERIOD	V. ROJAS AVE.		PR-2				TOTAL 15 MIN	TOTAL 60 MIN
	SB-L	SB-R	EB-L	EB-T	WB-T	WB-R		
12:00 AM	2	4	8	18	14	8	54	
12:15 AM	3	6	8	20	6	12	55	
12:30 AM	3	6	2	6	3	2	22	
12:45 AM	1	2	8	18	11	0	40	171
1:00 AM	1	3	6	15	6	2	33	150
1:15 AM	3	5	3	8	1	4	24	119
1:30 AM	1	1	5	11	2	0	20	117
1:45 AM	1	1	3	6	3	2	16	93
2:00 AM	2	4	5	11	2	4	28	88
2:15 AM	1	3	1	3	4	7	19	83
2:30 AM	1	3	4	9	6	7	30	93
2:45 AM	2	4	2	4	4	6	22	99
3:00 AM	4	6	3	7	1	4	25	96
3:15 AM	3	5	3	7	3	7	28	105
3:30 AM	4	7	4	9	3	10	37	112
3:45 AM	3	4	3	7	6	10	33	123
4:00 AM	7	11	5	11	7	16	57	155
4:15 AM	6	10	10	23	9	19	77	204
4:30 AM	10	16	4	10	10	24	74	241
4:45 AM	15	25	4	10	19	42	115	323
5:00 AM	18	31	12	27	18	26	132	398
5:15 AM	13	21	12	28	35	39	148	469
5:30 AM	20	34	13	31	48	67	213	608
5:45 AM	21	37	9	20	41	55	183	676
6:00 AM	24	40	10	24	60	57	215	759
6:15 AM	35	60	12	28	104	103	342	953
6:30 AM	39	67	17	39	195	238	595	1335
6:45 AM	58	100	15	35	155	197	560	1712
7:00 AM	54	93	21	50	272	239	729	2226
<b>7:15 AM</b>	50	84	26	62	344	290	856	2740
<b>7:30 AM</b>	49	84	28	64	256	166	647	2792
<b>7:45 AM</b>	44	74	29	68	250	173	638	2870
<b>8:00 AM</b>	44	74	35	82	282	228	745	<b>2886</b>
8:15 AM	44	74	35	82	216	179	630	2660
8:30 AM	43	73	29	67	216	191	619	2632
8:45 AM	59	71	36	85	169	134	554	2548
9:00 AM	46	56	31	71	194	172	570	2373
9:15 AM	64	78	40	92	118	138	530	2273
9:30 AM	67	82	45	105	146	122	567	2221
9:45 AM	59	73	45	106	122	101	506	2173
10:00 AM	68	62	44	104	166	153	597	2200
10:15 AM	66	61	50	116	150	121	564	2234
10:30 AM	79	73	46	108	123	128	557	2224
10:45 AM	82	76	59	136	110	115	578	2296
11:00 AM	79	72	51	119	136	114	571	2270
11:15 AM	92	84	52	122	140	124	614	2320
11:30 AM	86	80	49	115	122	125	577	2340
11:45 AM	76	70	50	118	121	92	527	2289
12:00 PM	96	78	56	129	137	126	622	2340
12:15 PM	88	72	52	122	155	121	610	2336
12:30 PM	82	67	59	136	164	127	635	2394

Peak Hour

TIME PERIOD	V. ROJAS AVE.		PR-2				TOTAL 15 MIN	TOTAL 60 MIN
	SB-L	SB-R	EB-L	EB-T	WB-T	WB-R		
12:45 PM	86	71	56	132	178	146	669	2536
1:00 PM	92	76	56	130	138	100	592	2506
1:15 PM	79	64	55	129	165	103	595	2491
1:30 PM	101	64	51	120	157	120	613	2469
1:45 PM	94	60	51	119	162	109	595	2395
2:00 PM	91	58	54	126	188	127	644	2447
<b>2:15 PM</b>	89	57	55	127	189	149	666	2518
<b>2:30 PM</b>	109	69	62	144	167	107	658	2563
<b>2:45 PM</b>	87	55	54	126	209	108	639	2607
<b>3:00 PM</b>	83	53	62	146	212	163	719	<b>2682</b>
3:15 PM	104	66	56	132	164	135	657	2673
3:30 PM	94	60	59	139	148	82	582	2597
3:45 PM	90	58	57	133	146	109	593	2551
4:00 PM	107	69	71	165	117	121	650	2482
4:15 PM	85	55	61	143	147	118	609	2434
4:30 PM	99	63	59	137	142	147	647	2499
4:45 PM	100	64	58	136	128	131	617	2523
5:00 PM	81	51	60	139	117	70	518	2391
5:15 PM	77	49	67	156	118	110	577	2359
5:30 PM	86	55	59	137	111	124	572	2284
5:45 PM	71	45	53	122	72	65	428	2095
6:00 PM	76	49	62	143	90	77	497	2074
6:15 PM	61	39	49	113	79	70	411	1908
6:30 PM	79	50	40	93	66	75	403	1739
6:45 PM	80	51	45	105	65	69	415	1726
7:00 PM	62	39	37	85	68	57	348	1577
7:15 PM	45	28	35	82	69	71	330	1496
7:30 PM	58	37	35	83	48	51	312	1405
7:45 PM	54	35	29	69	38	53	278	1268
8:00 PM	61	39	38	90	34	53	315	1235
8:15 PM	55	35	31	71	29	54	275	1180
8:30 PM	48	30	28	65	19	38	228	1096
8:45 PM	46	30	31	71	24	18	220	1038
9:00 PM	34	22	29	67	45	42	239	962
9:15 PM	39	25	26	60	33	50	233	920
9:30 PM	30	28	23	53	34	34	202	894
9:45 PM	21	20	21	48	29	17	156	830
10:00 PM	7	6	20	46	39	17	135	726
10:15 PM	9	9	22	51	49	40	180	673
10:30 PM	9	12	18	42	35	26	142	613
10:45 PM	12	14	22	50	11	14	123	580
11:00 PM	10	16	15	35	16	18	110	555
11:15 PM	8	12	17	38	27	25	127	502
11:30 PM	8	12	12	29	15	13	89	449
11:45 PM	5	8	13	30	18	25	99	425
<b>TOTAL</b>	<b>4640</b>	<b>4135</b>	<b>3073</b>	<b>7161</b>	<b>9040</b>	<b>7898</b>		

Peak Hour

**LOCATION: PR-2, EAST OF VICTOR ROSA AVENUE, ARECIBO**  
**DATE: WEDNESDAY, JANUARY 27, 2010**

TIME PERIOD	PR-2		TOTAL 15 MIN	TOTAL 60 MIN
	EB	WB		
12:00 AM	20	23	43	
12:15 AM	23	19	42	
12:30 AM	9	5	14	
12:45 AM	19	12	31	130
1:00 AM	16	8	24	111
1:15 AM	12	5	17	86
1:30 AM	12	2	14	86
1:45 AM	7	5	12	67
2:00 AM	13	6	19	62
2:15 AM	4	12	16	61
2:30 AM	10	14	24	71
2:45 AM	6	11	17	76
3:00 AM	11	5	16	73
3:15 AM	10	11	21	78
3:30 AM	13	14	27	81
3:45 AM	10	17	27	91
4:00 AM	18	24	42	117
4:15 AM	29	29	58	154
4:30 AM	20	36	56	183
4:45 AM	25	64	89	245
5:00 AM	44	46	90	293
5:15 AM	41	78	119	354
5:30 AM	51	121	172	470
5:45 AM	41	101	142	523
6:00 AM	48	123	171	604
6:15 AM	63	217	280	765
6:30 AM	78	455	533	1126
6:45 AM	93	370	463	1447
7:00 AM	104	537	641	1917
<b>7:15 AM</b>	112	666	778	2415
<b>7:30 AM</b>	113	443	556	2438
<b>7:45 AM</b>	112	444	556	2531
<b>8:00 AM</b>	126	536	662	<b>2552</b>
8:15 AM	126	415	541	2315
8:30 AM	110	427	537	2296
8:45 AM	144	318	462	2202
9:00 AM	117	384	501	2041
9:15 AM	156	269	425	1925
9:30 AM	172	281	453	1841
9:45 AM	165	234	399	1778
10:00 AM	172	335	507	1784
10:15 AM	182	285	467	1826
10:30 AM	187	264	451	1824
10:45 AM	217	236	453	1878
11:00 AM	198	263	461	1832
11:15 AM	214	277	491	1856
11:30 AM	201	259	460	1865
11:45 AM	195	224	419	1831
12:00 PM	225	276	501	1871
12:15 PM	210	290	500	1880

Peak Hour

12:30 PM	218	306	524	1944
12:45 PM	218	340	558	2083
1:00 PM	222	250	472	2054
1:15 PM	208	281	489	2043
1:30 PM	221	291	512	2031
1:45 PM	213	285	498	1971
2:00 PM	217	331	548	2047
<b>2:15 PM</b>	216	355	571	2129
<b>2:30 PM</b>	253	288	541	2158
<b>2:45 PM</b>	218	333	551	2211
<b>3:00 PM</b>	229	394	623	<b>2286</b>
3:15 PM	236	314	550	2265
3:30 PM	233	242	475	2199
3:45 PM	223	268	491	2139
4:00 PM	272	250	522	2038
4:15 PM	228	278	506	1994
4:30 PM	236	303	539	2058
4:45 PM	236	272	508	2075
5:00 PM	220	196	416	1969
5:15 PM	233	239	472	1935
5:30 PM	223	247	470	1866
5:45 PM	193	144	337	1695
6:00 PM	219	175	394	1673
6:15 PM	176	156	332	1533
6:30 PM	172	148	320	1383
6:45 PM	185	141	326	1372
7:00 PM	147	131	278	1256
7:15 PM	127	147	274	1198
7:30 PM	141	104	245	1123
7:45 PM	123	96	219	1016
8:00 PM	151	91	242	980
8:15 PM	126	87	213	919
8:30 PM	113	60	173	847
8:45 PM	117	44	161	789
9:00 PM	101	91	192	739
9:15 PM	99	87	186	712
9:30 PM	83	71	154	693
9:45 PM	69	48	117	649
10:00 PM	53	59	112	569
10:15 PM	60	93	153	536
10:30 PM	51	64	115	497
10:45 PM	62	26	88	468
11:00 PM	45	36	81	437
11:15 PM	46	55	101	385
11:30 PM	37	29	66	336
11:45 PM	38	45	83	331
<b>TOTAL</b>	<b>11811</b>	<b>17787</b>		

Peak Hour

**TELPEG ENGINEERING**  
TRAFFIC COUNTING SPECIALISTS

C: 787-366-1352 E: TELPEG@GMAIL.COM



# APPENDIX – C

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**DATA SUMMARY**

**INTERSECTION: PR-2 & PR-10 AND JUAN ROSADO AVE., ARECIBO, P.R.  
DATE: WEDNESDAY, JANUARY 27, 2010**

PEAK HOUR (A.M.)	PR-10				JUAN ROSADO AVE.				PR-2				TOTAL 15 MIN	
	NB-L	NB-T	NB-R	SB-R	SB-L	SB-T	SB-R	EB-L	EB-T	EB-R	WB-L	WB-T		WB-R
	7:15 - 7:30 AM	104	34	35	16	10	19	16	12	41	47	156		233
7:30 - 7:45 AM	124	41	37	18	10	23	18	12	43	46	124	185	31	694
7:45 - 8:00 AM	105	35	39	26	11	31	26	14	46	56	118	177	29	687
8:00 - 8:15 AM	84	28	47	18	13	21	18	13	55	54	130	194	32	689
PEAK HOUR VOLUME (A.M.)	417	138	158	78	44	94	78	51	185	203	528	789	131	
PEAK HOUR FACTOR (A.M.)	0.84	0.84	0.84	0.75	0.85	0.76	0.75	0.91	0.84	0.91	0.85	0.85	0.84	0.84

PEAK HOUR (P.M.)	PR-10				JUAN ROSADO AVE.				PR-2				TOTAL 15 MIN	
	NB-L	NB-T	NB-R	SB-R	SB-L	SB-T	SB-R	EB-L	EB-T	EB-R	WB-L	WB-T		WB-R
	2:15 - 2:30 PM	86	28	73	27	20	32	27	22	86	86	90		134
2:30 - 2:45 PM	65	21	82	30	23	37	30	23	97	90	86	129	21	704
2:45 - 3:00 PM	52	17	72	24	20	29	24	27	85	108	96	144	24	698
3:00 - 3:15 PM	59	20	83	22	23	27	22	26	98	104	96	145	24	727
PEAK HOUR VOLUME (P.M.)	262	86	310	103	86	125	103	98	366	388	368	552	91	
PEAK HOUR FACTOR (P.M.)	0.76	0.77	0.93	0.86	0.93	0.84	0.86	0.91	0.93	0.90	0.96	0.95	0.95	0.95

24 HOUR VOLUME	PR-10				JUAN ROSADO AVE.				PR-2				
	NB-L	NB-T	NB-R	SB-R	SB-L	SB-T	SB-R	EB-L	EB-T	EB-R	WB-L	WB-T	WB-R
	3928	1287	4089	1193	1130	1466	1193	1249	4814	4988	4794	7189	1192

**TELPEG ENGINEERING**  
TRAFFIC COUNTING SPECIALISTS

O: 787-366-1352 E: TELPEG@GMAIL.COM

**DATA SUMMARY**

**INTERSECTION: PR-2 & VICTOR ROJAS AVENUE, ARECIBO, P.R.  
DATE: WEDNESDAY, JANUARY 27, 2010**

PEAK HOUR (A.M.)	V. ROJAS AVE.		PR-2				TOTAL 15 MIN
	SB-L	SB-R	EB-L	EB-T	WB-T	WB-R	
7:15 - 7:30 AM	50	84	26	62	344	290	856
7:30 - 7:45 AM	49	84	28	64	256	166	647
7:45 - 8:00 AM	44	74	29	68	250	173	638
8:00 - 8:15 AM	44	74	35	82	282	228	745
PEAK HOUR VOLUME (A.M.)	187	316	118	276	1132	857	
PEAK HOUR FACTOR (A.M.)	0.94	0.94	0.84	0.84	0.82	0.74	

PEAK HOUR (P.M.)	V. ROJAS AVE.		PR-2				TOTAL 15 MIN
	SB-L	SB-R	EB-L	EB-T	WB-T	WB-R	
2:15 - 2:30 PM	89	57	55	127	189	149	666
2:30 - 2:45 PM	109	69	62	144	167	107	658
2:45 - 3:00 PM	87	55	54	126	209	108	639
3:00 - 3:15 PM	83	53	62	146	212	163	719
PEAK HOUR VOLUME (P.M.)	368	234	233	543	777	527	
PEAK HOUR FACTOR (P.M.)	0.84	0.85	0.94	0.93	0.92	0.81	

24 HOUR VOLUME	V. ROJAS AVE.		PR-2			
	SB-L	SB-R	EB-L	EB-T	WB-T	WB-R
	4640	4135	3073	7161	9040	7898

**TELPEG ENGINEERING**  
TRAFFIC COUNTING SPECIALISTS

C: 787-366-1352 E: TELPEG@GMAIL.COM

**DATA SUMMARY**

**LOCATION: PR-2, EAST OF VICTOR ROJAS AVENUE**  
**DATE: WEDNESDAY, JANUARY 27, 2010**

PEAK HOUR (A.M.)	PR-2		TOTAL 15 MIN
	EB	WB	
7:15 - 7:30 AM	112	666	778
7:30 - 7:45 AM	113	443	556
7:45 - 8:00 AM	112	444	556
8:00 - 8:15 AM	126	536	662
PEAK HOUR VOLUME (A.M.)	463	2089	
PEAK HOUR FACTOR (A.M.)	0.92	0.78	

PEAK HOUR (P.M.)	PR-2		TOTAL 15 MIN
	EB	WB	
2:15 - 2:30 PM	216	355	571
2:30 - 2:45 PM	253	288	541
2:45 - 3:00 PM	218	333	551
3:00 - 3:15 PM	229	394	623
PEAK HOUR VOLUME (P.M.)	916	1370	
PEAK HOUR FACTOR (P.M.)	0.91	0.87	

24 HOUR VOLUME	PR-2	
	EB	WB
	11811	17787



# APPENDIX – D

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Arecibo  
 Summary of Trip Generation Calculation  
 For 150 Employees of General Light Industrial  
 March 20, 2010

	Average Rate	Standard Deviation	Adjustment Factor	Driveway Volume
Avg. Weekday 2-Way Volume	3.02	1.86	1.00	453
7-9 AM Peak Hour Enter	0.37	0.00	1.00	56
7-9 AM Peak Hour Exit	0.07	0.00	1.00	11
7-9 AM Peak Hour Total	0.44	0.69	1.00	66
4-6 PM Peak Hour Enter	0.09	0.00	1.00	14
4-6 PM Peak Hour Exit	0.33	0.00	1.00	50
4-6 PM Peak Hour Total	0.42	0.67	1.00	63
Saturday 2-Way Volume	0.48	0.72	1.00	72
Saturday Peak Hour Enter	0.02	0.00	1.00	3
Saturday Peak Hour Exit	0.03	0.00	1.00	5
Saturday Peak Hour Total	0.05	0.23	1.00	8

Note: A zero indicates no data available.  
 Source: Institute of Transportation Engineers  
 Trip Generation, 7th Edition, 2003.

TRIP GENERATION BY MICROTRANS

Arecibo  
 Summary of Trip Generation Calculation  
 For 150 Employees of General Light Industrial  
 March 20, 2010

	Average Rate	Standard Deviation	Adjustment Factor	Driveway Volume
Avg. Weekday 2-Way Volume	3.02	1.86	1.00	453
7-9 AM Peak Hour Enter	0.37	0.00	1.00	56
7-9 AM Peak Hour Exit	0.07	0.00	1.00	11
7-9 AM Peak Hour Total	0.44	0.69	1.00	66
4-6 PM Peak Hour Enter	0.09	0.00	1.00	14
4-6 PM Peak Hour Exit	0.33	0.00	1.00	50
4-6 PM Peak Hour Total	0.42	0.67	1.00	63
AM Pk Hr, Generator, Enter	0.42	0.00	1.00	63
AM Pk Hr, Generator, Exit	0.06	0.00	1.00	9
AM Pk Hr, Generator, Total	0.48	0.72	1.00	72
PM Pk Hr, Generator, Enter	0.15	0.00	1.00	23
PM Pk Hr, Generator, Exit	0.36	0.00	1.00	54
PM Pk Hr, Generator, Total	0.51	0.75	1.00	77
Saturday 2-Way Volume	0.48	0.72	1.00	72
Saturday Peak Hour Enter	0.02	0.00	1.00	3
Saturday Peak Hour Exit	0.03	0.00	1.00	5
Saturday Peak Hour Total	0.05	0.23	1.00	8
Sunday 2-Way Volume	0.26	0.60	1.00	39
Sunday Peak Hour Enter	0.02	0.00	1.00	3
Sunday Peak Hour Exit	0.02	0.00	1.00	3
Sunday Peak Hour Total	0.04	0.20	1.00	6

Note: A zero indicates no data available.  
 Source: Institute of Transportation Engineers  
 Trip Generation, 7th Edition, 2003.

TRIP GENERATION BY MICROTRANS

Arecibo  
 Summary of Trip Generation Rates  
 For 150 Employees of General Light Industrial  
 March 20, 2010

	Avg. Trip Rate	Min. Trip Rate	Max. Trip Rate	Std. Dev.	No. of Studies
Avg. Weekday 2-Way Volume	3.02	1.53	4.48	1.86	18
7-9 AM Peak Hour Enter	0.37	0.07	0.85	0.00	0
7-9 AM Peak Hour Exit	0.07	0.01	0.17	0.00	0
7-9 AM Peak Hour Total	0.44	0.08	1.02	0.69	21
4-6 PM Peak Hour Enter	0.09	0.01	0.20	0.00	0
4-6 PM Peak Hour Exit	0.33	0.03	0.75	0.00	0
4-6 PM Peak Hour Total	0.42	0.04	0.95	0.67	19
AM Pk Hr, Generator, Enter	0.42	0.22	0.89	0.00	0
AM Pk Hr, Generator, Exit	0.06	0.03	0.13	0.00	0
AM Pk Hr, Generator, Total	0.48	0.25	1.02	0.72	21
PM Pk Hr, Generator, Enter	0.15	0.10	0.34	0.00	0
PM Pk Hr, Generator, Exit	0.36	0.26	0.84	0.00	0
PM Pk Hr, Generator, Total	0.51	0.36	1.18	0.75	21
Saturday 2-Way Volume	0.48	0.29	1.32	0.72	6
Saturday Peak Hour Enter	0.02	0.02	0.10	0.00	0
Saturday Peak Hour Exit	0.03	0.02	0.11	0.00	0
Saturday Peak Hour Total	0.05	0.04	0.21	0.23	5
Sunday 2-Way Volume	0.26	0.12	2.09	0.60	4
Sunday Peak Hour Enter	0.02	0.01	0.14	0.00	0
Sunday Peak Hour Exit	0.02	0.01	0.15	0.00	0
Sunday Peak Hour Total	0.04	0.02	0.29	0.20	4

Note: A zero indicates no data available.  
 Source: Institute of Transportation Engineers  
 Trip Generation, 7th Edition, 2003.

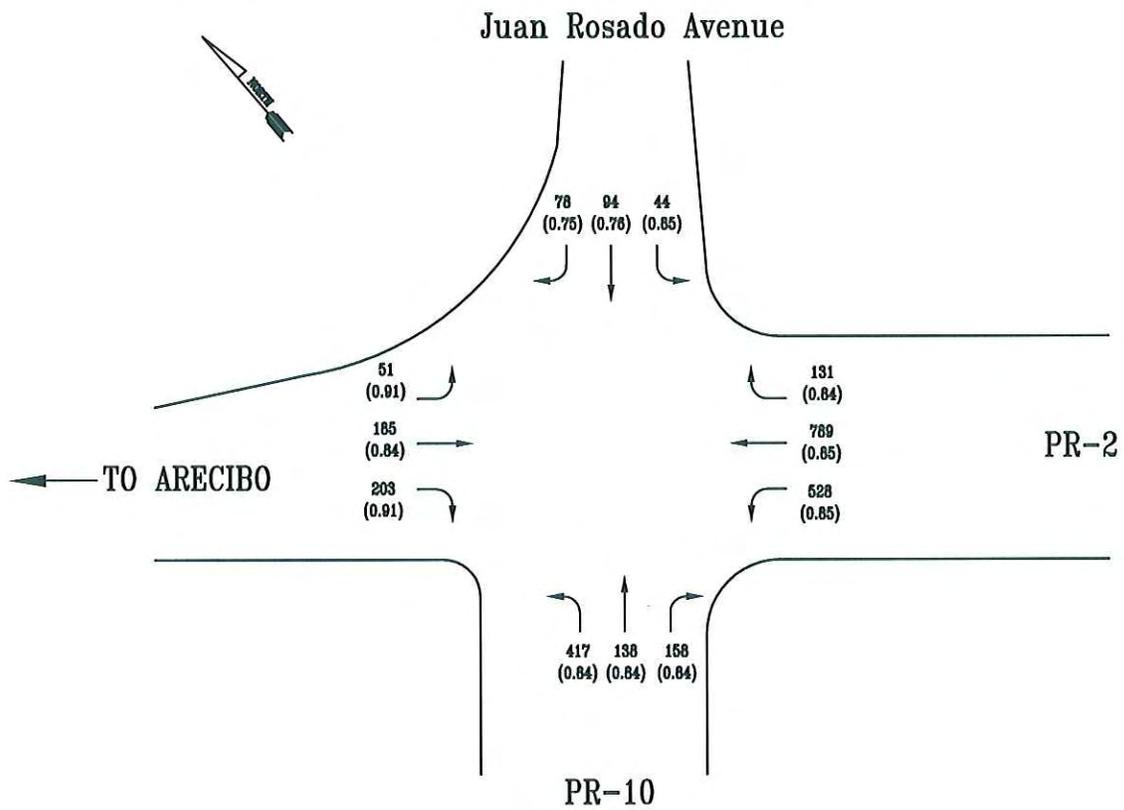
TRIP GENERATION BY MICROTRANS



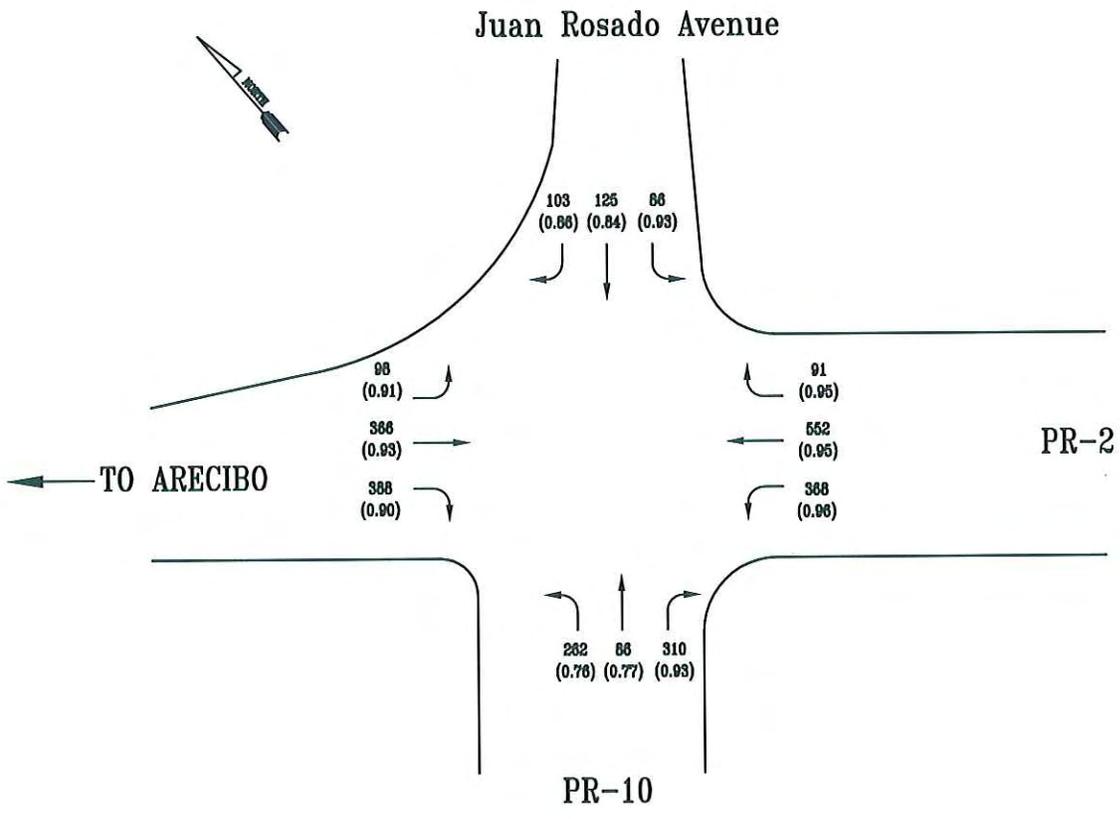
# APPENDIX – E

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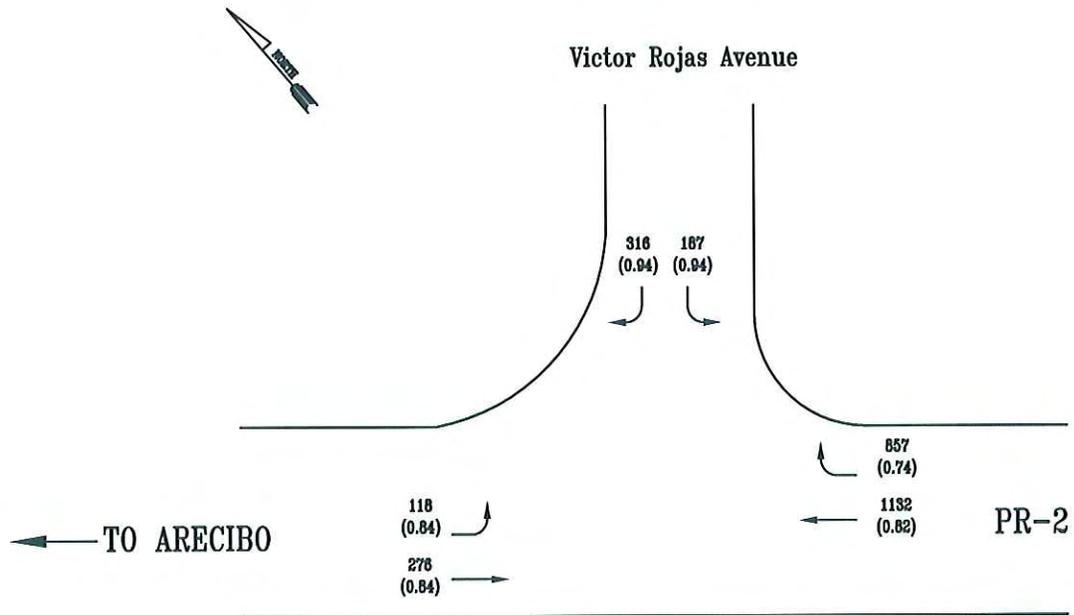
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 Existing AM Peak Hour Volumes and Peak Hour Factors



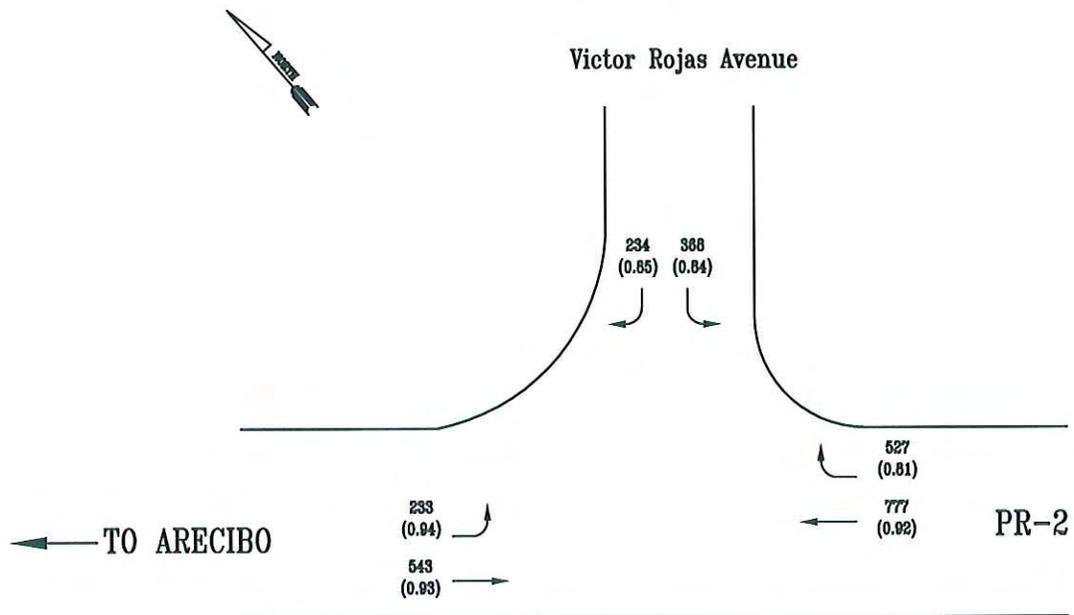
INTERSECTION #1: PR-2 & PR-10 AND JUAN ROSADO AVENUE  
 Existing PM Peak Hour Volumes and Peak Hour Factors



INTERSECTION #2: PR-2 & VICTOR ROJAS AVENUE  
Existing AM Peak Hour Volumes and Peak Hour Factors



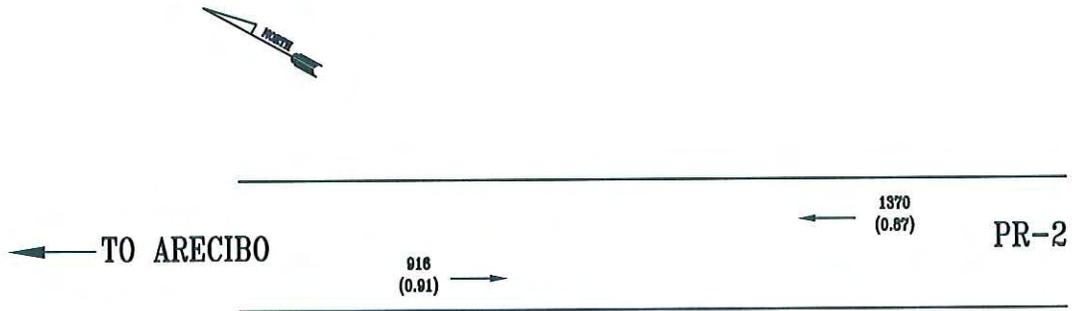
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Existing PM Peak Hour Volumes and Peak Hour Factors



PR-2 KM. 73.1  
Existing AM Peak Hour Volumes and Peak Hour Factors



PR-2 KM. 73.1  
Existing PM Peak Hour Volumes and Peak Hour Factors

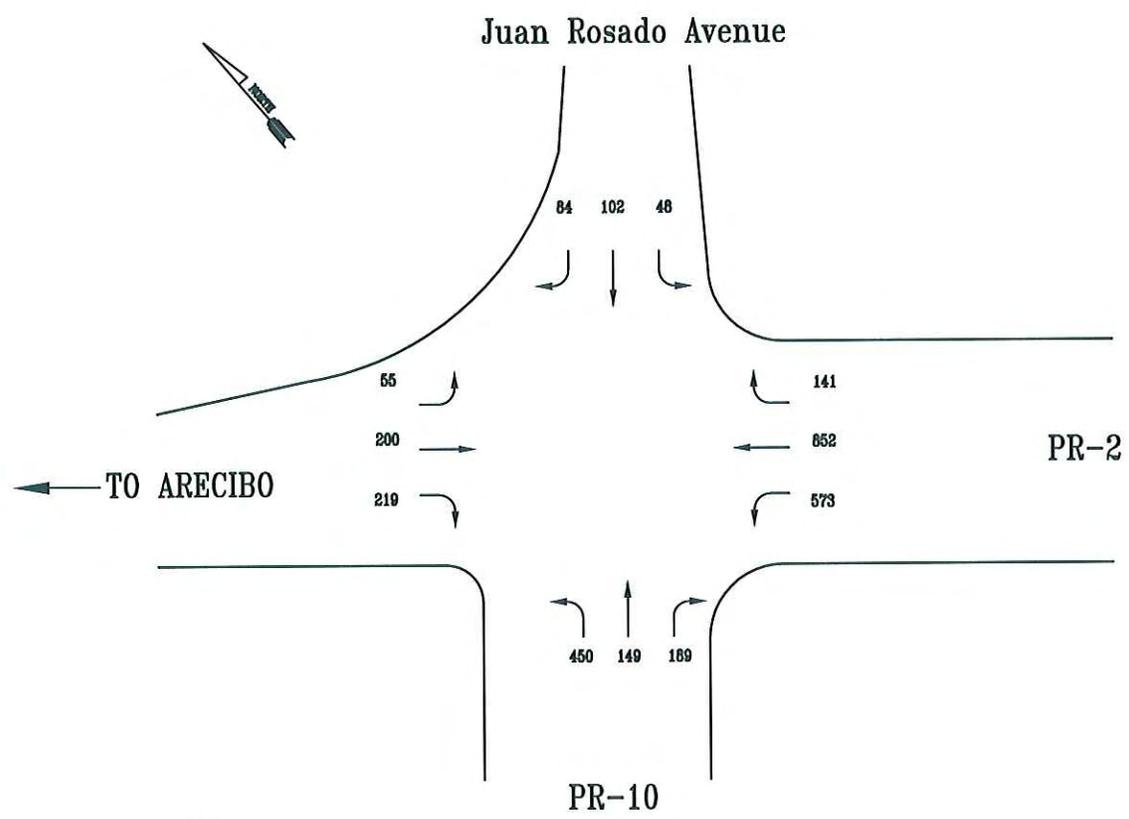




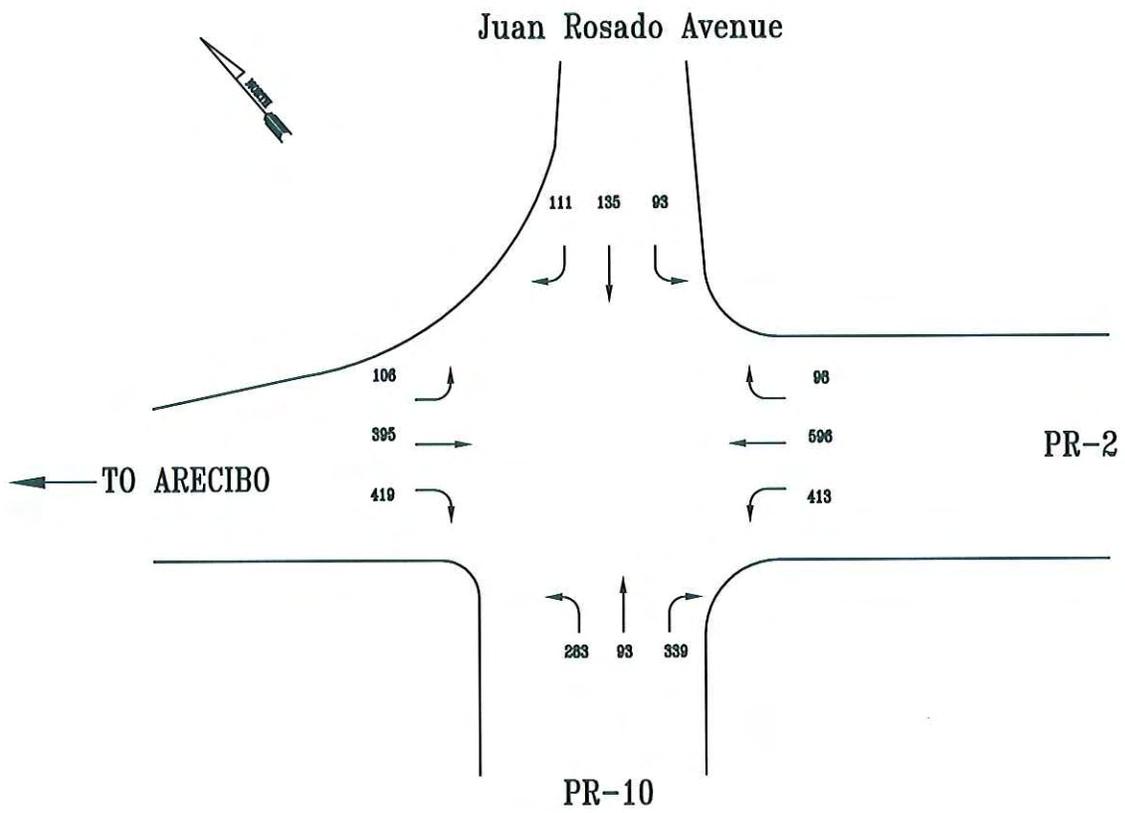
# APPENDIX – F

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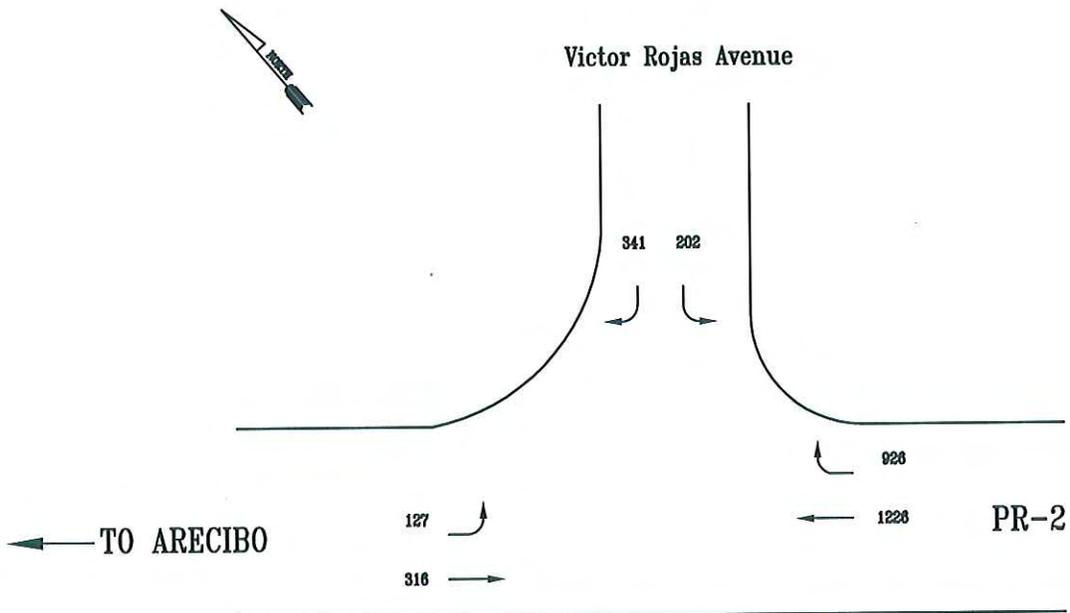
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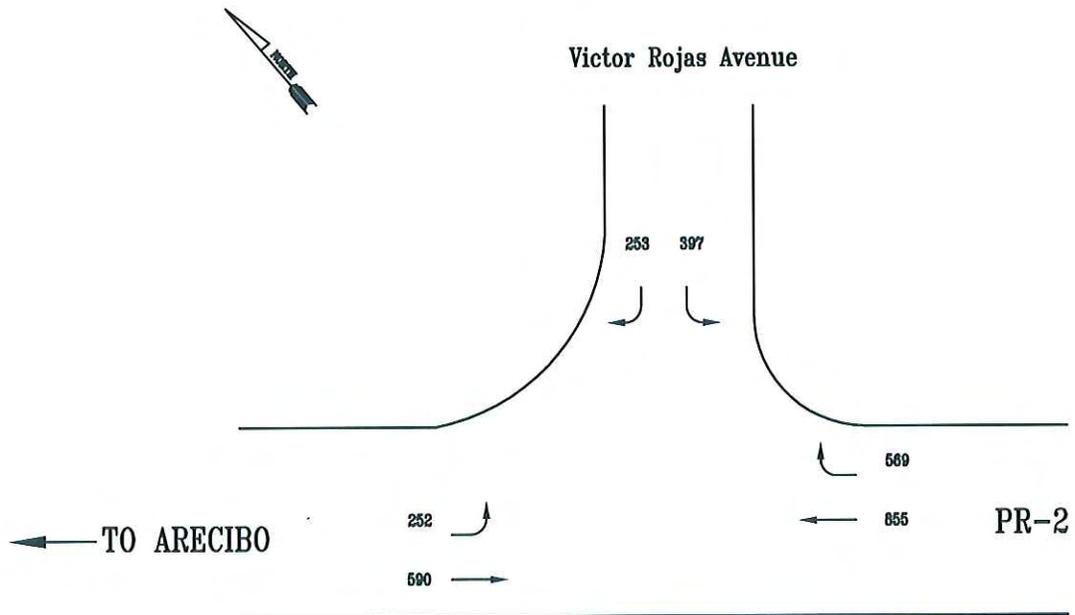
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2013 PM Peak Hour Volumes



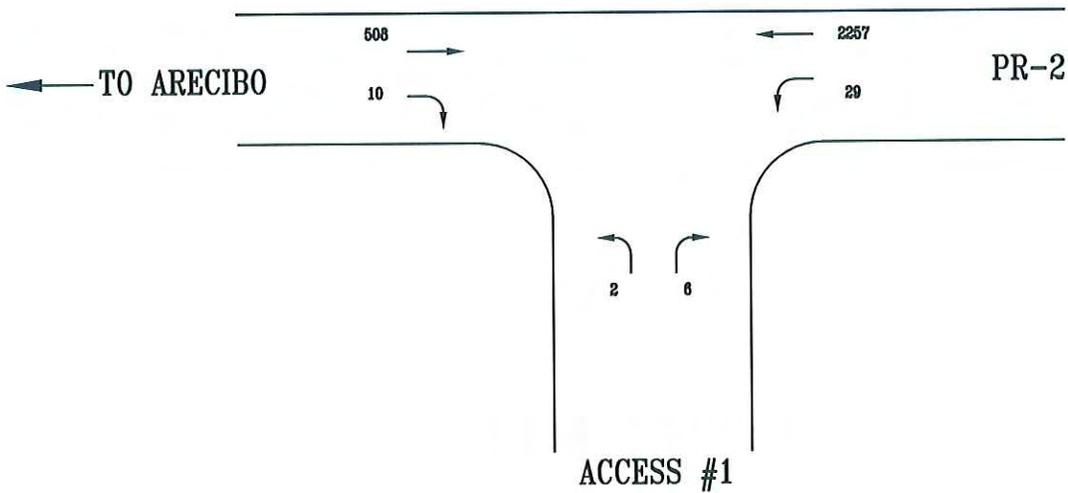
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2013 AM Peak Hour Volumes



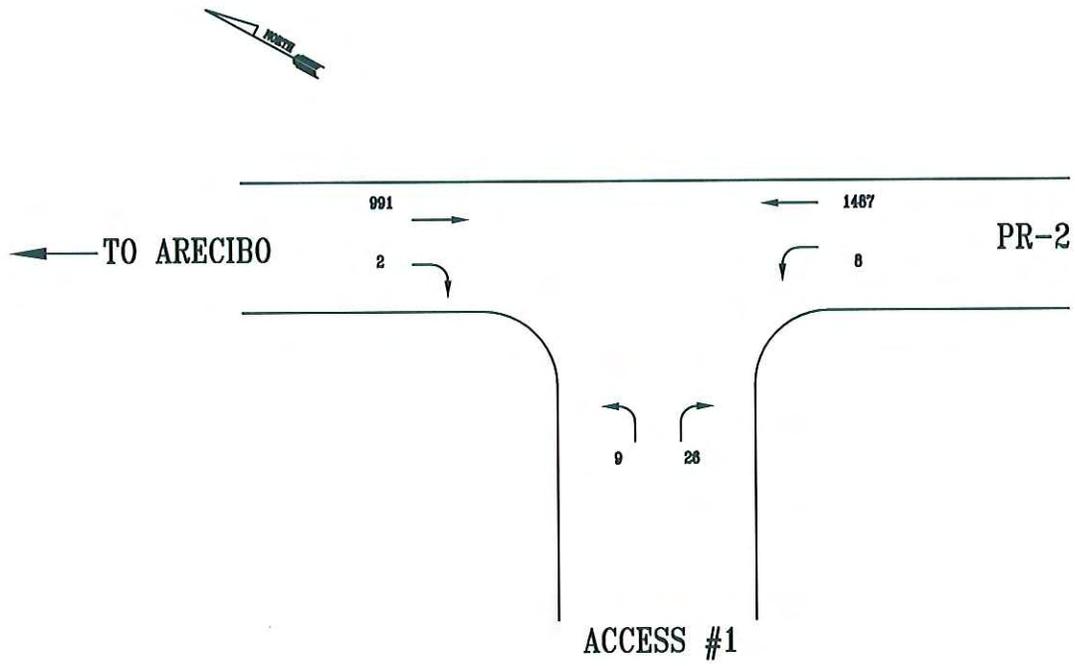
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2013 PM Peak Hour Volumes



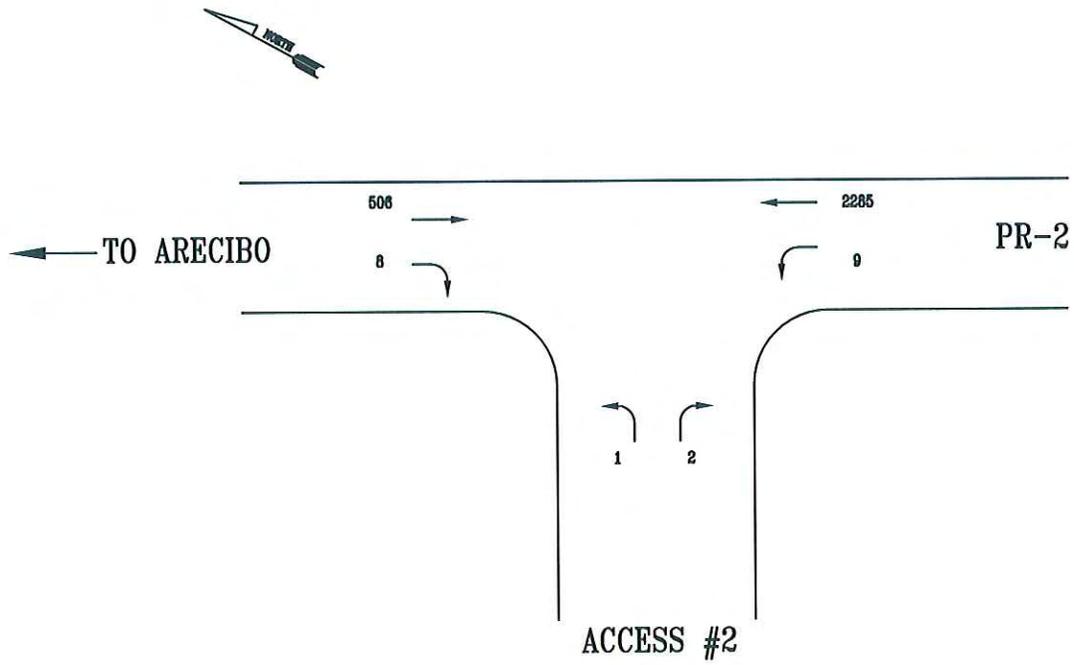
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2013 AM Peak Hour Volumes



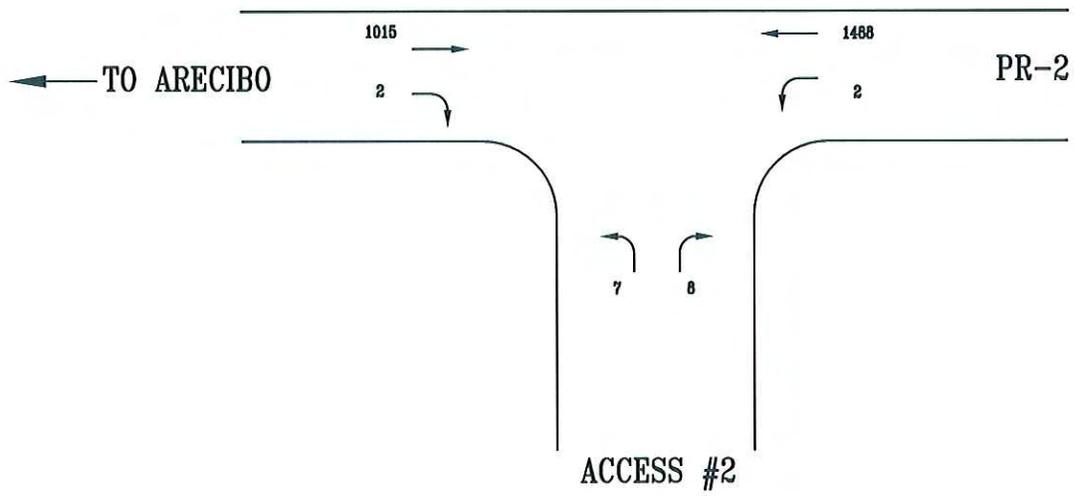
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2013 PM Peak Hour Volumes



INTERSECTION #4: PR-2 AND ACCESS #2  
2013 AM Peak Hour Volumes



INTERSECTION #4: PR-2 AND ACCESS #2  
2013 PM Peak Hour Volumes

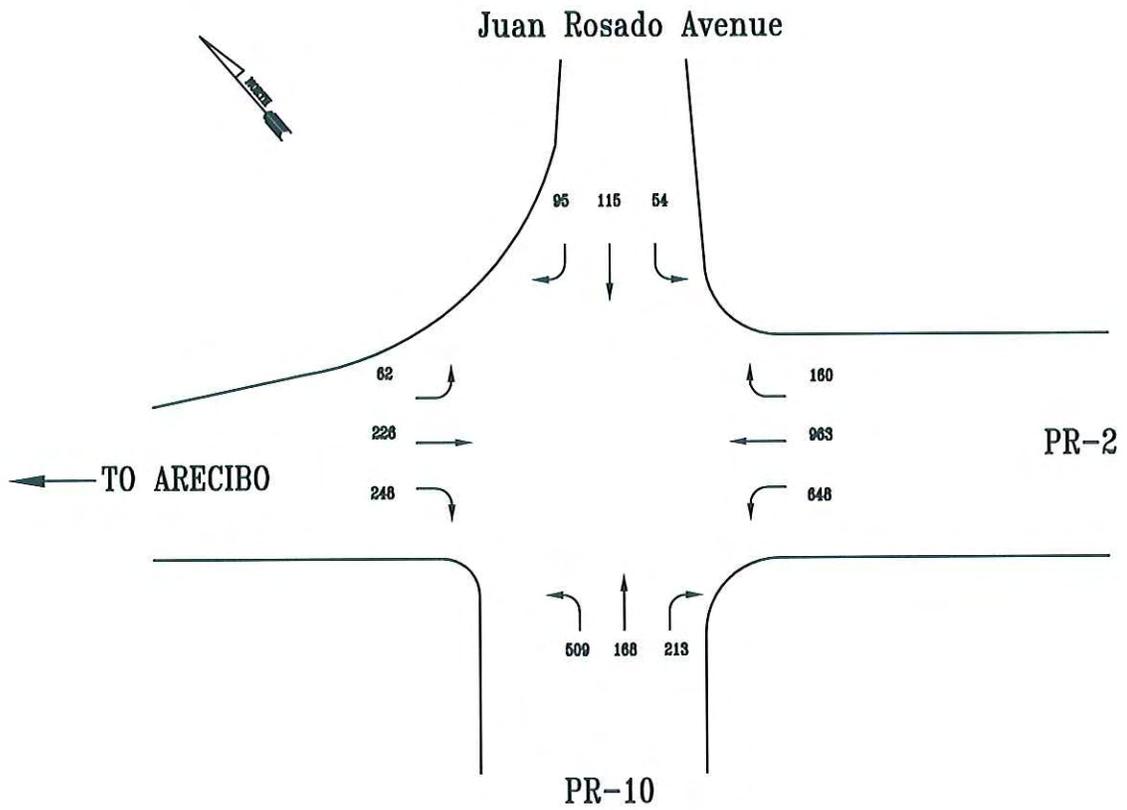




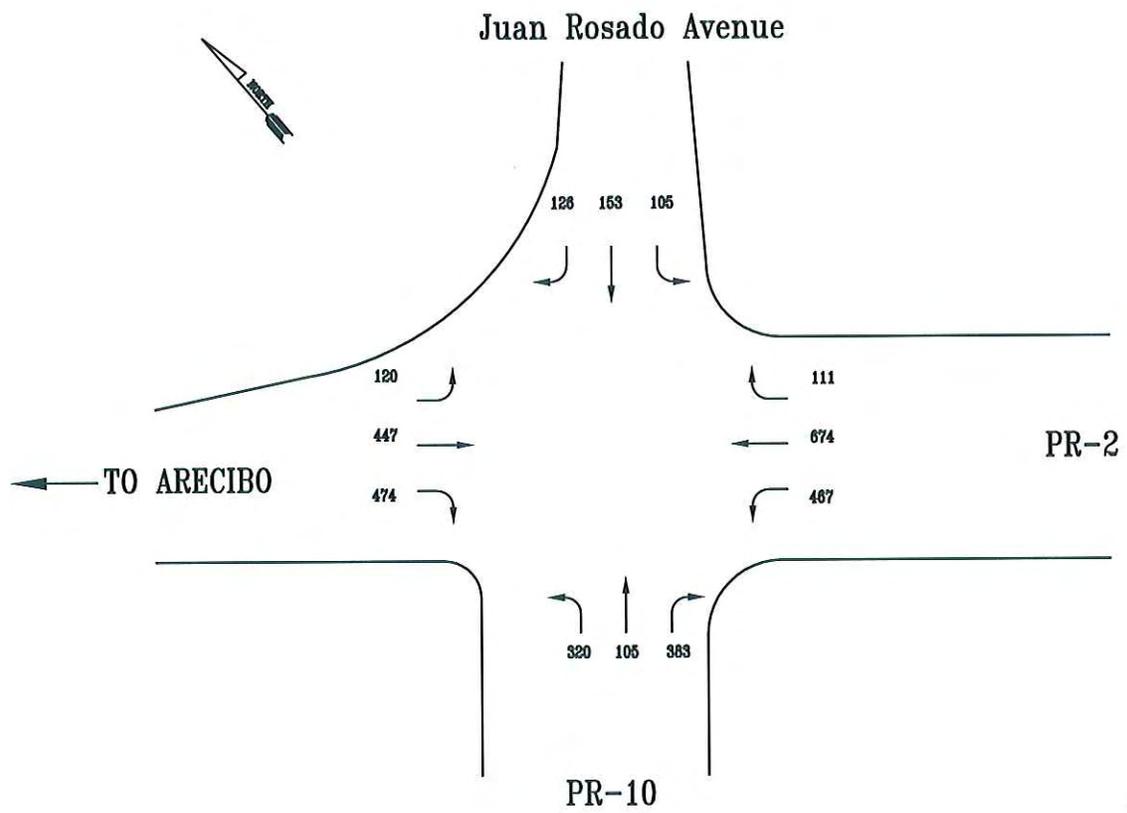
# APPENDIX – G

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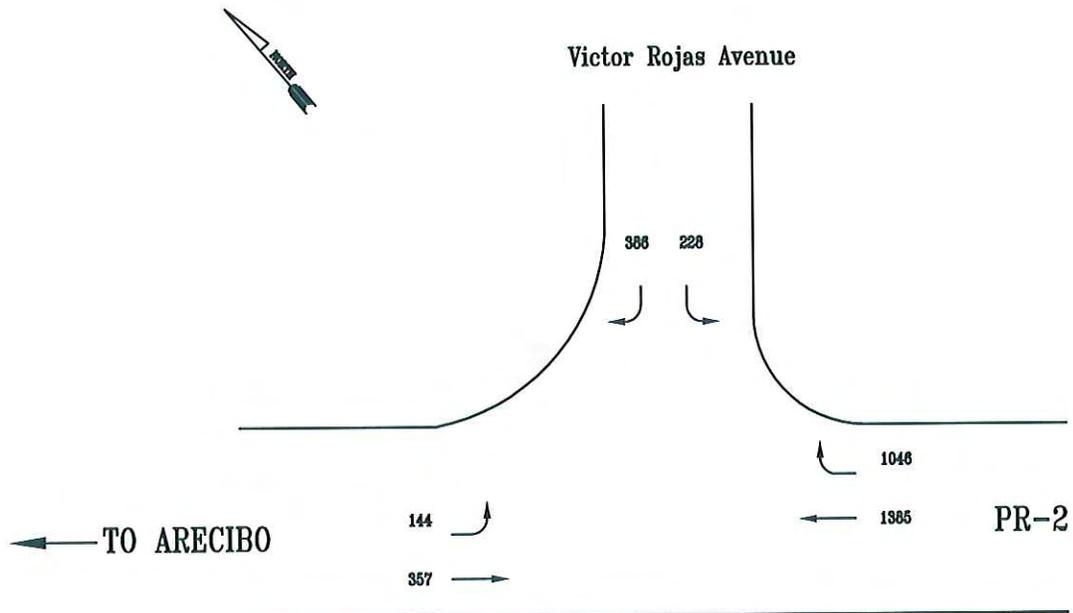
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2018 AM Peak Hour Volumes



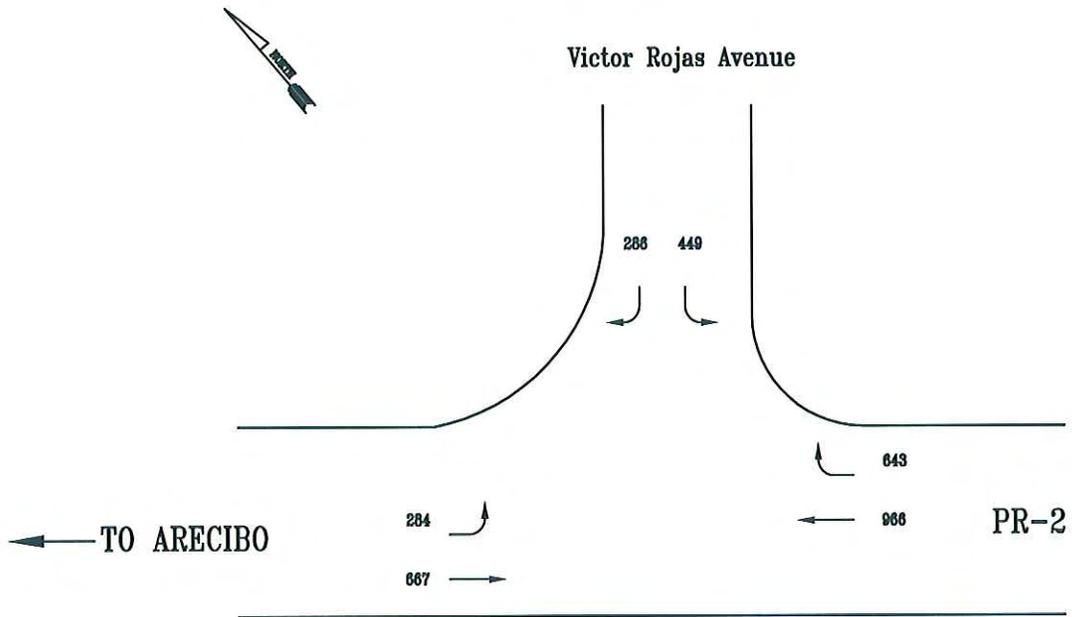
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2018 PM Peak Hour Volumes



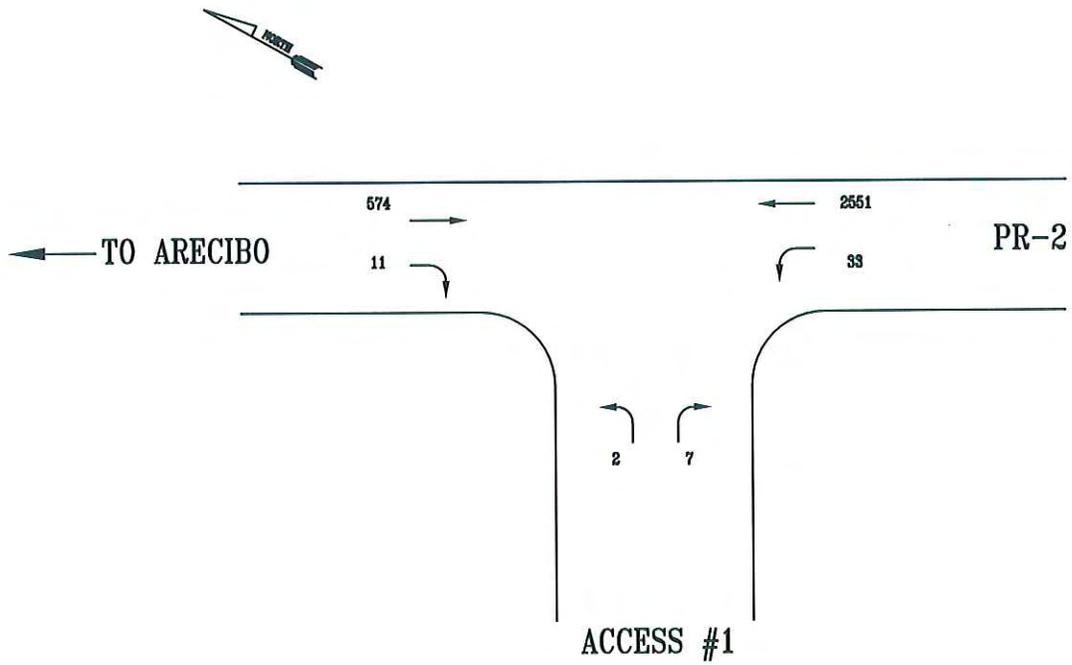
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2018 AM Peak Hour Volumes



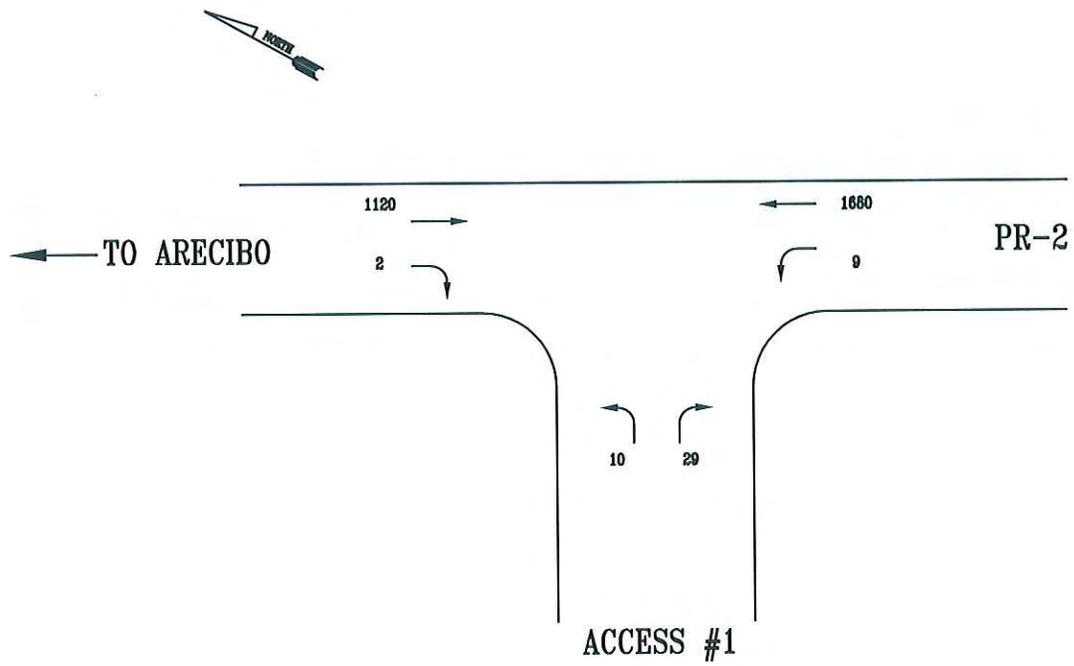
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2018 PM Peak Hour Volumes



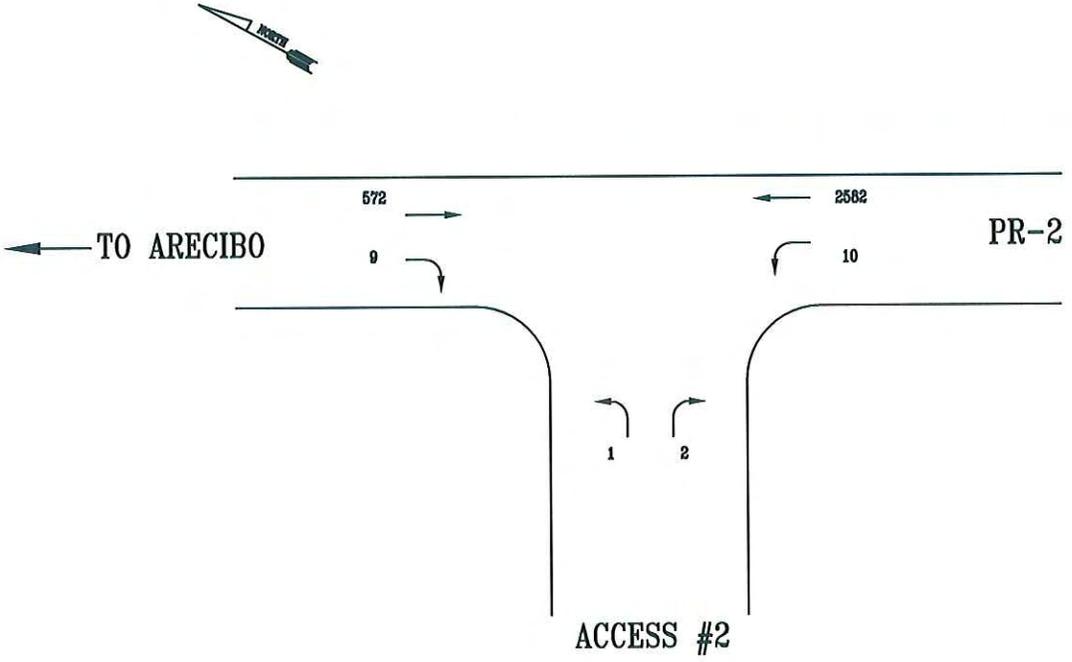
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2018 AM Peak Hour Volumes



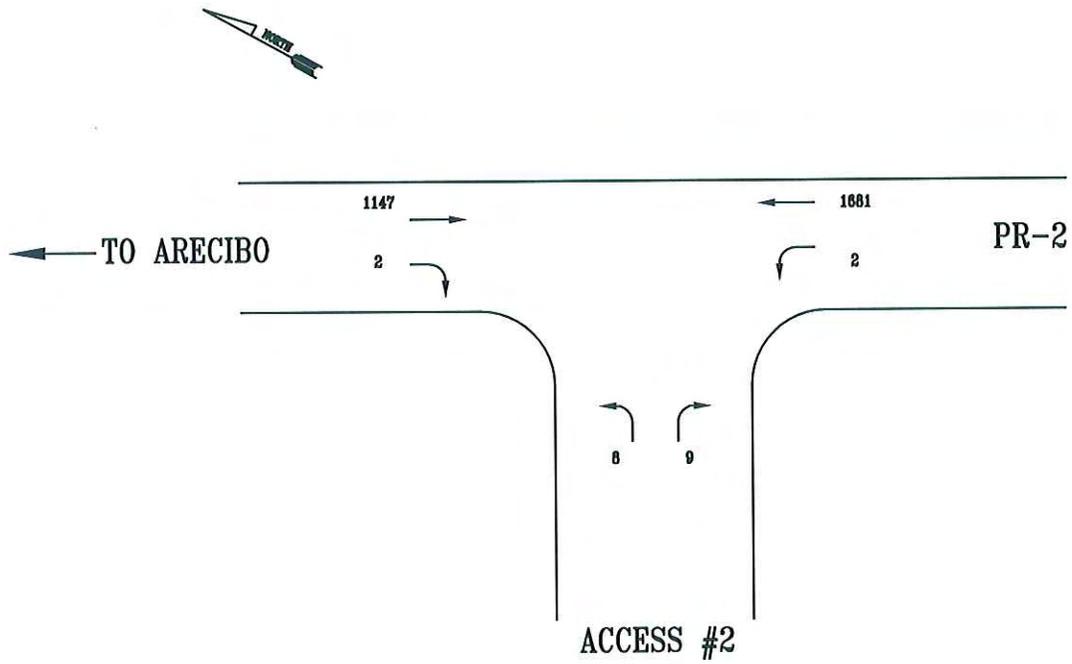
INTERSECTION #3: PR-2 AND ACCESS #1  
2018 PM Peak Hour Volumes



INTERSECTION #4: PR-2 AND ACCESS #2  
2018 AM Peak Hour Volumes



INTERSECTION #4: PR-2 AND ACCESS #2  
2018 PM Peak Hour Volumes





# APPENDIX – H

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Arecibo Resource Recovery Facility  
Existing AM Peak

Traffic Study  
3/25/2010

Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations	↔	↕↕		↕↕	↕↕			↕↕			↕↕	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	11	11	9	11	11	12	11	12	12	11	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	48		0	85		0	0		0	0		0
Storage Lanes	1		0	2		0	0		0	0		0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Leading Detector (ft)	48	50		50	50		50	50		50	50	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	1.00	0.95	0.95	0.97	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor												
Frt		0.925			0.978			0.967			0.944	
Flt Protected	0.950			0.950				0.972			0.991	
Satd. Flow (prot)	1652	3165	0	3090	3346	0	0	3216	0	0	3201	0
Flt Permitted	0.950			0.950				0.972			0.991	
Satd. Flow (perm)	1652	3165	0	3090	3346	0	0	3216	0	0	3201	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		187			14			23			65	
Headway Factor	1.09	1.04	1.04	1.14	1.04	1.04	1.00	1.04	1.00	1.00	1.04	1.00
Link Speed (mph)		35			45			35			25	
Link Distance (ft)		1093			4064			1070			1297	
Travel Time (s)		21.3			61.6			20.8			35.4	
Volume (vph)	51	185	203	528	789	131	417	138	158	44	94	78
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.91	0.84	0.91	0.85	0.85	0.84	0.84	0.84	0.84	0.85	0.76	0.75
Growth Factor	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	60	236	239	665	993	167	531	176	201	55	132	111
Lane Group Flow (vph)	60	475	0	665	1160	0	0	908	0	0	298	0
Turn Type	Prot			Prot			Split			Split		
Protected Phases	1	6		5	2		4	4		8	8	
Permitted Phases												
Detector Phases	1	6		5	2		4	4		8	8	
Minimum Initial (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Minimum Split (s)	8.5	20.5		8.5	20.5		20.5	20.5		20.0	20.0	
Total Split (s)	24.5	59.5	0.0	24.5	59.5	0.0	44.5	44.5	0.0	24.5	24.5	0.0
Total Split (%)	16.0%	38.9%	0.0%	16.0%	38.9%	0.0%	29.1%	29.1%	0.0%	16.0%	16.0%	0.0%
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	1.5	1.5		1.5	1.5		1.5	1.5		1.5	1.5	
Lead/Lag	Lead	Lag		Lead	Lag							
Lead-Lag Optimize?	Yes	Yes		Yes	Yes							
Recall Mode	None	Min		None	Min		None	None		None	None	
Act Effect Green (s)	10.1	36.5		20.5	49.7			41.0			14.7	
Actuated g/C Ratio	0.08	0.28		0.16	0.38			0.31			0.11	

Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
v/c Ratio	0.48	0.47		1.38	0.91			0.96dl			0.72	
Control Delay	64.9	22.7		223.2	45.7			55.9			49.5	
Queue Delay	0.0	0.0		0.0	0.0			0.0			0.0	
Total Delay	64.9	22.7		223.2	45.7			55.9			49.5	
LOS	E	C		F	D			E			D	
Approach Delay		27.5			110.4			55.9			49.5	
Approach LOS		C			F			E			D	

Intersection Summary

Area Type: Other  
 Cycle Length: 153  
 Actuated Cycle Length: 131.2  
 Natural Cycle: 100  
 Control Type: Actuated-Uncoordinated  
 Maximum v/c Ratio: 1.38  
 Intersection Signal Delay: 79.0  
 Intersection Capacity Utilization 77.7%  
 Analysis Period (min) 15  
 dl Defacto Left Lane. Recode with 1 though lane as a left lane.

Intersection LOS: E  
 ICU Level of Service D

Splits and Phases: 1: PR-2 & J Rosado Ave

ø1	ø2	ø4	ø8
24.5 s	59.5 s	44.5 s	24.5 s
ø5	ø6		
24.5 s	59.5 s		

Arecibo Resource Recovery Facility  
Existing AM Peak

Traffic Study  
3/25/2010

Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)		0%	0%		0%	
Storage Length (ft)	220			0	0	0
Storage Lanes	1			0	2	0
Total Lost Time (s)	4.0	5.0	5.0	5.0	5.0	5.0
Leading Detector (ft)	50	50	50		50	
Trailing Detector (ft)	0	0	0		0	
Turning Speed (mph)	15			9	15	9
Lane Util. Factor	1.00	0.95	0.95	0.95	0.97	0.95
Ped Bike Factor						
Frt			0.932		0.906	
Flt Protected	0.950				0.982	
Satd. Flow (prot)	1770	3539	3299	0	3215	0
Flt Permitted	0.950				0.982	
Satd. Flow (perm)	1770	3539	3299	0	3215	0
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)			180		360	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Link Speed (mph)		45	45		35	
Link Distance (ft)		4064	4074		1144	
Travel Time (s)		61.6	61.7		22.3	
Volume (vph)	118	276	1132	857	187	316
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.84	0.84	0.82	0.74	0.94	0.94
Growth Factor	107%	107%	107%	107%	107%	107%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Adj. Flow (vph)	150	352	1477	1239	213	360
Lane Group Flow (vph)	150	352	2716	0	573	0
Turn Type	Prot					
Protected Phases	5	2	6		4	
Permitted Phases						
Detector Phases	5	2	6		4	
Minimum Initial (s)	4.0	4.0	4.0		4.0	
Minimum Split (s)	8.0	21.0	21.0		21.0	
Total Split (s)	24.0	64.0	40.0	0.0	55.0	0.0
Total Split (%)	20.2%	53.8%	33.6%	0.0%	46.2%	0.0%
Yellow Time (s)	2.0	3.0	3.0		3.0	
All-Red Time (s)	2.0	2.0	2.0		2.0	
Lead/Lag	Lead		Lag			
Lead-Lag Optimize?	Yes		Yes			
Recall Mode	None	Min	Min		None	
Act Effct Green (s)	11.1	48.5	36.0		10.8	
Actuated g/C Ratio	0.16	0.70	0.52		0.16	



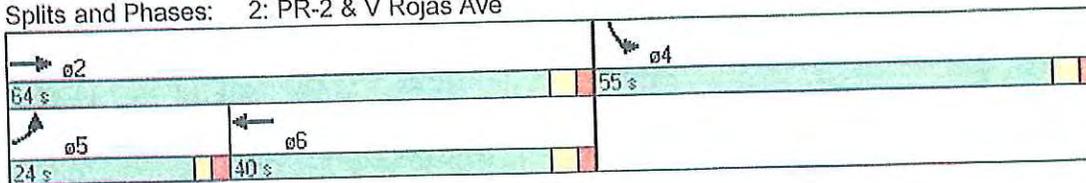
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
v/c Ratio	0.55	0.14	1.51		0.71	
Control Delay	30.9	4.1	253.7		11.3	
Queue Delay	0.0	0.0	0.0		0.0	
Total Delay	30.9	4.1	253.7		11.3	
LOS	C	A	F		B	
Approach Delay		12.1	253.7		11.3	
Approach LOS		B	F		B	

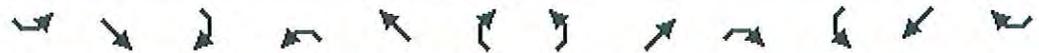
Intersection Summary

Area Type: Other  
 Cycle Length: 119  
 Actuated Cycle Length: 69.6  
 Natural Cycle: 150  
 Control Type: Actuated-Uncoordinated  
 Maximum v/c Ratio: 1.51  
 Intersection Signal Delay: 185.1  
 Intersection Capacity Utilization 98.0%  
 Analysis Period (min) 15

Intersection LOS: F  
 ICU Level of Service F

Splits and Phases: 2: PR-2 & V Rojas Ave





Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations												
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	11	11	9	11	11	12	11	12	12	11	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	48		0	85		0	0		0	0		0
Storage Lanes	1		0	2		0	0		0	0		0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Leading Detector (ft)	48	50		50	50		50	50		50	50	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	1.00	0.95	0.95	0.97	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor												
Frt		0.925			0.978			0.964			0.944	
Flt Protected	0.950			0.950				0.972			0.991	
Satd. Flow (prot)	1652	3165	0	3090	3346	0	0	3169	0	0	3201	0
Flt Permitted	0.950			0.950				0.972			0.991	
Satd. Flow (perm)	1652	3165	0	3090	3346	0	0	3169	0	0	3201	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		186			14			26			64	
Headway Factor	1.09	1.04	1.04	1.14	1.04	1.04	1.00	1.04	1.00	1.00	1.04	1.00
Link Speed (mph)		35			45			35			25	
Link Distance (ft)		1093			4064			1070			1297	
Travel Time (s)		21.3			61.6			20.8			35.4	
Volume (vph)	55	200	219	573	852	141	450	149	189	48	102	84
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.91	0.84	0.91	0.85	0.85	0.84	0.84	0.84	0.85	0.85	0.76	0.75
Growth Factor	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	7%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	65	255	258	721	1073	180	573	190	238	60	144	120
Lane Group Flow (vph)	65	513	0	721	1253	0	0	1001	0	0	324	0
Turn Type	Prot			Prot			Split			Split		
Protected Phases	1	6		5	2		4	4		8	8	
Permitted Phases												
Detector Phases	1	6		5	2		4	4		8	8	
Minimum Initial (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Minimum Split (s)	8.5	20.5		8.5	20.5		20.5	20.5		20.0	20.0	
Total Split (s)	24.5	59.5	0.0	24.5	59.5	0.0	44.5	44.5	0.0	24.5	24.5	0.0
Total Split (%)	16.0%	38.9%	0.0%	16.0%	38.9%	0.0%	29.1%	29.1%	0.0%	16.0%	16.0%	0.0%
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	1.5	1.5		1.5	1.5		1.5	1.5		1.5	1.5	
Lead/Lag	Lead	Lag		Lead	Lag							
Lead-Lag Optimize?	Yes	Yes		Yes	Yes							
Recall Mode	None	Min		None	Min		None	None		None	None	
Act Effct Green (s)	10.7	43.5		20.1	55.3			40.2			16.0	
Actuated g/C Ratio	0.08	0.32		0.15	0.40			0.29			0.12	



Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
v/c Ratio	0.51	0.46		1.60	0.93			1.11dl			0.76	
Control Delay	66.4	24.2		317.2	52.6			92.8			53.9	
Queue Delay	0.0	0.0		0.0	0.0			0.0			0.0	
Total Delay	66.4	24.2		317.2	52.6			92.8			53.9	
LOS	E	C		F	D			F			D	
Approach Delay		28.9			149.3			92.8			53.9	
Approach LOS		C			F			F			D	

**Intersection Summary**

Area Type: Other  
 Cycle Length: 153  
 Actuated Cycle Length: 138  
 Natural Cycle: 120  
 Control Type: Actuated-Uncoordinated  
 Maximum v/c Ratio: 1.60  
 Intersection Signal Delay: 108.8      Intersection LOS: F  
 Intersection Capacity Utilization 82.4%      ICU Level of Service E  
 Analysis Period (min) 15  
 dl Defacto Left Lane. Recode with 1 though lane as a left lane.

Splits and Phases: 1: PR-2 & J Rosado Ave

ø1	ø2	ø4	ø8
24.5 s	59.5 s	44.5 s	24.5 s
ø5	ø6		
24.5 s	59.5 s		



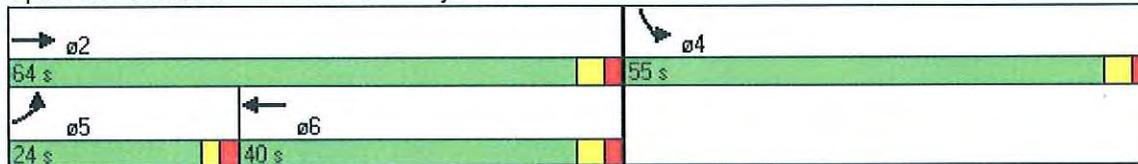
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↔	↕↕	↕↕		↔↔	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)		0%	0%		0%	
Storage Length (ft)	220			0	0	0
Storage Lanes	1			0	2	0
Total Lost Time (s)	4.0	5.0	5.0	5.0	5.0	5.0
Leading Detector (ft)	50	50	50		50	
Trailing Detector (ft)	0	0	0		0	
Turning Speed (mph)	15			9	15	9
Lane Util. Factor	1.00	0.95	0.95	0.95	0.97	0.95
Ped Bike Factor						
Frnt			0.932		0.906	
Flt Protected	0.950				0.982	
Satd. Flow (prot)	1770	3438	3299	0	3215	0
Flt Permitted	0.950				0.982	
Satd. Flow (perm)	1770	3438	3299	0	3215	0
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)			179		388	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Link Speed (mph)		45	45		35	
Link Distance (ft)		4064	3282		1144	
Travel Time (s)		61.6	49.7		22.3	
Volume (vph)	127	316	1226	926	202	341
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.84	0.85	0.82	0.74	0.94	0.94
Growth Factor	107%	107%	107%	107%	107%	107%
Heavy Vehicles (%)	2%	5%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Adj. Flow (vph)	162	398	1600	1339	230	388
Lane Group Flow (vph)	162	398	2939	0	618	0
Turn Type	Prot					
Protected Phases	5	2	6		4	
Permitted Phases						
Detector Phases	5	2	6		4	
Minimum Initial (s)	4.0	4.0	4.0		4.0	
Minimum Split (s)	8.0	21.0	21.0		21.0	
Total Split (s)	24.0	64.0	40.0	0.0	55.0	0.0
Total Split (%)	20.2%	53.8%	33.6%	0.0%	46.2%	0.0%
Yellow Time (s)	2.0	3.0	3.0		3.0	
All-Red Time (s)	2.0	2.0	2.0		2.0	
Lead/Lag	Lead		Lag			
Lead-Lag Optimize?	Yes		Yes			
Recall Mode	None	Min	Min		None	
Act Effct Green (s)	11.6	49.1	36.2		11.4	
Actuated g/C Ratio	0.16	0.69	0.51		0.16	



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
v/c Ratio	0.57	0.17	1.66		0.73	
Control Delay	31.7	4.4	318.1		11.4	
Queue Delay	0.0	0.0	0.0		0.0	
Total Delay	31.7	4.4	318.1		11.4	
LOS	C	A	F		B	
Approach Delay		12.3	318.1		11.4	
Approach LOS		B	F		B	

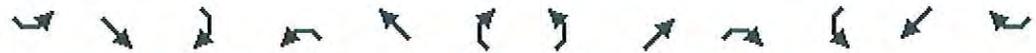
Intersection Summary	
Area Type:	Other
Cycle Length:	119
Actuated Cycle Length:	70.8
Natural Cycle:	150
Control Type:	Actuated-Uncoordinated
Maximum v/c Ratio:	1.66
Intersection Signal Delay:	230.5
Intersection Capacity Utilization	105.0%
Analysis Period (min)	15
Intersection LOS:	F
ICU Level of Service	G

Splits and Phases: 2: PR-2 & V Rojas Ave



							
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations				 	 		
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Volume (veh/h)	2	6	29	2257	508	10	
Peak Hour Factor	1.00	1.00	1.00	0.78	0.92	1.00	
Hourly flow rate (vph)	2	6	31	3096	591	11	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None						
Median storage veh							
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume	2201	295	602				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	2201	295	602				
tC, single (s)	8.8	8.9	6.1				
tC, 2 stage (s)							
tF (s)	4.5	4.3	3.2				
p0 queue free %	79	99	94				
cM capacity (veh/h)	10	479	524				
Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3
Volume Total	9	31	1548	1548	295	295	11
Volume Left	2	31	0	0	0	0	0
Volume Right	6	0	0	0	0	0	11
cSH	38	524	1700	1700	1700	1700	1700
Volume to Capacity	0.22	0.06	0.91	0.91	0.17	0.17	0.01
Queue Length 95th (ft)	18	5	0	0	0	0	0
Control Delay (s)	124.2	12.3	0.0	0.0	0.0	0.0	0.0
Lane LOS	F	B					
Approach Delay (s)	124.2	0.1			0.0		
Approach LOS	F						
Intersection Summary							
Average Delay			0.4				
Intersection Capacity Utilization			76.8%		ICU Level of Service		D
Analysis Period (min)			15				

Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations							
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Volume (veh/h)	1	2	9	2285	506	8	
Peak Hour Factor	1.00	1.00	1.00	0.79	0.92	1.00	
Hourly flow rate (vph)	1	2	10	3095	588	9	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	Raised						
Median storage veh	0						
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume	2155	294	597				
vC1, stage 1 conf vol	588						
vC2, stage 2 conf vol	1567						
vCu, unblocked vol	2155	294	597				
tC, single (s)	6.8	6.9	4.1				
tC, 2 stage (s)	5.8						
tF (s)	3.5	3.3	2.2				
p0 queue free %	99	100	99				
cM capacity (veh/h)	91	702	976				
Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3
Volume Total	3	10	1547	1547	294	294	9
Volume Left	1	10	0	0	0	0	0
Volume Right	2	0	0	0	0	0	9
cSH	216	976	1700	1700	1700	1700	1700
Volume to Capacity	0.01	0.01	0.91	0.91	0.17	0.17	0.01
Queue Length 95th (ft)	1	1	0	0	0	0	0
Control Delay (s)	21.9	8.7	0.0	0.0	0.0	0.0	0.0
Lane LOS	C	A					
Approach Delay (s)	21.9	0.0			0.0		
Approach LOS	C						
Intersection Summary							
Average Delay			0.0				
Intersection Capacity Utilization			77.6%		ICU Level of Service		D
Analysis Period (min)			15				



Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations												
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	11	11	9	11	11	10	10	10	12	11	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	48		0	85		0	0		60	0		0
Storage Lanes	1		0	2		0	0		1	0		0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Leading Detector (ft)	48	50		50	50		50	50	50	50	50	
Trailing Detector (ft)	0	0		0	0		0	0	0	0	0	
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	1.00	0.95	0.95	0.97	0.95	0.95	0.95	0.95	1.00	0.95	0.95	0.95
Ped Bike Factor												
Frt		0.925			0.978				0.850		0.944	
Flt Protected	0.950			0.950				0.964			0.991	
Satd. Flow (prot)	1652	3165	0	3090	3346	0	0	3184	1409	0	3201	0
Flt Permitted	0.950			0.950				0.964			0.991	
Satd. Flow (perm)	1652	3165	0	3090	3346	0	0	3184	1409	0	3201	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		162			14				78		57	
Headway Factor	1.09	1.04	1.04	1.14	1.04	1.04	1.09	1.09	1.09	1.00	1.04	1.00
Link Speed (mph)		45			45			35			25	
Link Distance (ft)		1093			4064			1070			1297	
Travel Time (s)		16.6			61.6			20.8			35.4	
Volume (vph)	55	200	219	573	852	141	450	149	189	48	102	84
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.91	0.84	0.91	0.85	0.85	0.84	0.84	0.84	0.85	0.85	0.76	0.75
Growth Factor	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	7%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	65	255	258	721	1073	180	573	190	238	60	144	120
Lane Group Flow (vph)	65	513	0	721	1253	0	0	763	238	0	324	0
Turn Type	Prot			Prot			Split		pt+ov	Split		
Protected Phases	1	6		5	2		4	4	4 5	8	8	
Permitted Phases												
Detector Phases	1	6		5	2		4	4	4 5	8	8	
Minimum Initial (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Minimum Split (s)	8.5	20.5		8.5	20.5		20.5	20.5		20.0	20.0	
Total Split (s)	24.5	59.5	0.0	39.0	74.0	0.0	44.5	44.5	83.5	24.5	24.5	0.0
Total Split (%)	14.6%	35.5%	0.0%	23.3%	44.2%	0.0%	26.6%	26.6%	49.9%	14.6%	14.6%	0.0%
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	1.5	1.5		1.5	1.5		1.5	1.5		1.5	1.5	
Lead/Lag	Lead	Lag		Lead	Lag							
Lead-Lag Optimize?	Yes	Yes		Yes	Yes							
Recall Mode	None	Min		None	Min		None	None		None	None	
Act Effct Green (s)	10.9	34.5		35.3	61.6			39.4	74.6		16.3	
Actuated g/C Ratio	0.07	0.24		0.25	0.43			0.27	0.52		0.11	



Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
v/c Ratio	0.52	0.58		0.95	0.87			1.27dl	0.31		0.78	
Control Delay	72.0	32.5		77.1	43.4			61.9	9.8		59.8	
Queue Delay	0.0	0.0		0.0	0.0			0.0	0.0		0.0	
Total Delay	72.0	32.5		77.1	43.4			61.9	9.8		59.8	
LOS	E	C		E	D			E	A		E	
Approach Delay		36.9			55.7			49.5			59.8	
Approach LOS		D			E			D			E	

Intersection Summary

Area Type: Other  
 Cycle Length: 167.5  
 Actuated Cycle Length: 143.8  
 Natural Cycle: 100  
 Control Type: Actuated-Uncoordinated  
 Maximum v/c Ratio: 0.95  
 Intersection Signal Delay: 51.6      Intersection LOS: D  
 Intersection Capacity Utilization 82.4%      ICU Level of Service E  
 Analysis Period (min) 15  
 dl Defacto Left Lane. Recode with 1 though lane as a left lane.

Splits and Phases: 1: PR-2 & J Rosado Ave

ø1 24.5 s	ø2 74 s	ø4 44.5 s	ø8 24.5 s
ø5 39 s	ø6 59.5 s		



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)		0%	0%		0%	
Storage Length (ft)	220			400	0	0
Storage Lanes	1			1	2	0
Total Lost Time (s)	4.0	5.0	5.0	5.0	5.0	5.0
Leading Detector (ft)	50	50	50	50	50	
Trailing Detector (ft)	0	0	0	0	0	
Turning Speed (mph)	15			9	15	9
Lane Util. Factor	1.00	0.95	0.95	1.00	0.97	0.95
Ped Bike Factor						
Fr <sub>t</sub>				0.850	0.906	
Fl <sub>t</sub> Protected	0.950				0.982	
Satd. Flow (prot)	1770	3438	3539	1583	3215	0
Fl <sub>t</sub> Permitted	0.950				0.982	
Satd. Flow (perm)	1770	3438	3539	1583	3215	0
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				818	388	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Link Speed (mph)		45	45		35	
Link Distance (ft)		4064	3282		1144	
Travel Time (s)		61.6	49.7		22.3	
Volume (vph)	127	316	1226	926	202	341
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.84	0.85	0.82	0.74	0.94	0.94
Growth Factor	107%	107%	107%	107%	107%	107%
Heavy Vehicles (%)	2%	5%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Adj. Flow (vph)	162	398	1600	1339	230	388
Lane Group Flow (vph)	162	398	1600	1339	618	0
Turn Type	Prot			Free		
Protected Phases	5	2	6		4	
Permitted Phases				Free		
Detector Phases	5	2	6		4	
Minimum Initial (s)	4.0	4.0	4.0		4.0	
Minimum Split (s)	8.0	21.0	21.0		21.0	
Total Split (s)	24.0	64.0	40.0	0.0	55.0	0.0
Total Split (%)	20.2%	53.8%	33.6%	0.0%	46.2%	0.0%
Yellow Time (s)	2.0	3.0	3.0		3.0	
All-Red Time (s)	2.0	2.0	2.0		2.0	
Lead/Lag	Lead		Lag			
Lead-Lag Optimize?	Yes		Yes			
Recall Mode	None	Min	Min		None	
Act Effct Green (s)	11.6	49.1	36.2	70.8	11.4	
Actuated g/C Ratio	0.16	0.69	0.51	1.00	0.16	

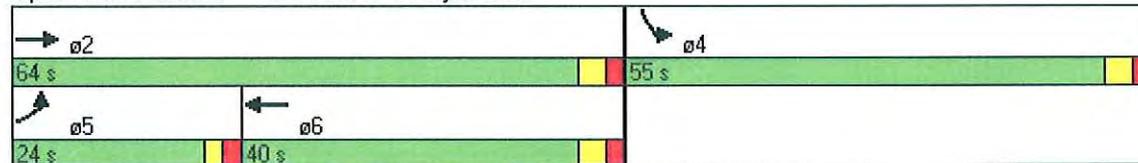


Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
v/c Ratio	0.57	0.17	0.88	0.85	0.73	
Control Delay	31.7	4.4	27.1	6.3	11.4	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	31.7	4.4	27.1	6.3	11.4	
LOS	C	A	C	A	B	
Approach Delay		12.3	17.6		11.4	
Approach LOS		B	B		B	

**Intersection Summary**

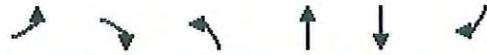
Area Type:	Other
Cycle Length:	119
Actuated Cycle Length:	70.8
Natural Cycle:	75
Control Type:	Actuated-Uncoordinated
Maximum v/c Ratio:	0.88
Intersection Signal Delay:	16.0
Intersection Capacity Utilization	73.2%
Analysis Period (min)	15
Intersection LOS:	B
ICU Level of Service	D

Splits and Phases: 2: PR-2 & V Rojas Ave





Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)	0%			0%	0%	
Storage Length (ft)	0	0	350			400
Storage Lanes	1	0	1			1
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Leading Detector (ft)	50		50	50	50	50
Trailing Detector (ft)	0		0	0	0	0
Turning Speed (mph)	15	9	15			9
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	1.00
Ped Bike Factor						
Frt	0.899					0.850
Flt Protected	0.988		0.950			
Satd. Flow (prot)	844	0	902	3539	3539	808
Flt Permitted	0.988		0.950			
Satd. Flow (perm)	844	0	902	3539	3539	808
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)	6					11
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Link Speed (mph)	15			45	45	
Link Distance (ft)	773			848	3282	
Travel Time (s)	35.1			12.8	49.7	
Volume (vph)	2	6	29	2257	508	10
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	1.00	1.00	1.00	0.78	0.92	1.00
Growth Factor	107%	107%	107%	107%	107%	107%
Heavy Vehicles (%)	100%	100%	100%	2%	2%	100%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Adj. Flow (vph)	2	6	31	3096	591	11
Lane Group Flow (vph)	8	0	31	3096	591	11
Turn Type			Prot			Free
Protected Phases	4		5	2	6	
Permitted Phases						Free
Detector Phases	4		5	2	6	
Minimum Initial (s)	10.0		4.0	4.0	4.0	
Minimum Split (s)	25.0		15.0	40.0	25.0	
Total Split (s)	25.0	0.0	20.0	45.0	25.0	0.0
Total Split (%)	35.7%	0.0%	28.6%	64.3%	35.7%	0.0%
Yellow Time (s)	3.0		3.0	3.0	3.0	
All-Red Time (s)	1.5		1.5	1.5	1.5	
Lead/Lag			Lead		Lag	
Lead-Lag Optimize?			Yes		Yes	
Recall Mode	None		None	Max	Max	
Act Effct Green (s)	11.4		10.0	89.4	79.8	94.8
Actuated g/C Ratio	0.11		0.10	0.94	0.84	1.00

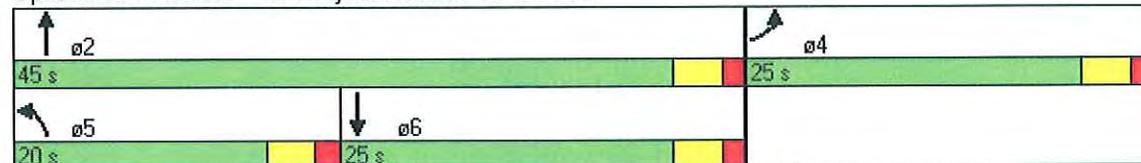


Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
v/c Ratio	0.08		0.35	0.93	0.20	0.01
Control Delay	15.1		22.7	11.9	4.0	0.0
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	15.1		22.7	11.9	4.0	0.0
LOS	B		C	B	A	A
Approach Delay	15.1			12.0	3.9	
Approach LOS	B			B	A	

**Intersection Summary**

Area Type:	Other
Cycle Length:	70
Actuated Cycle Length:	94.8
Natural Cycle:	150
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.93
Intersection Signal Delay:	10.7
Intersection Capacity Utilization	81.8%
Analysis Period (min)	15
Intersection LOS:	B
ICU Level of Service	D

Splits and Phases: 3: Project Access #1 & PR-2

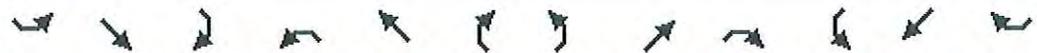




Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↙		↙	↑↑	↑↑	↘
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Volume (veh/h)	1	2	9	2285	506	8
Peak Hour Factor	1.00	1.00	1.00	0.79	0.92	1.00
Hourly flow rate (vph)	1	2	10	3095	588	9
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	Raised					
Median storage veh	0					
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	2155	294	597			
vC1, stage 1 conf vol	588					
vC2, stage 2 conf vol	1567					
vCu, unblocked vol	2155	294	597			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)	5.8					
tF (s)	3.5	3.3	2.2			
p0 queue free %	99	100	99			
cM capacity (veh/h)	91	702	976			

Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3
Volume Total	3	10	1547	1547	294	294	9
Volume Left	1	10	0	0	0	0	0
Volume Right	2	0	0	0	0	0	9
cSH	216	976	1700	1700	1700	1700	1700
Volume to Capacity	0.01	0.01	0.91	0.91	0.17	0.17	0.01
Queue Length 95th (ft)	1	1	0	0	0	0	0
Control Delay (s)	21.9	8.7	0.0	0.0	0.0	0.0	0.0
Lane LOS	C	A					
Approach Delay (s)	21.9	0.0			0.0		
Approach LOS	C						

Intersection Summary			
Average Delay		0.0	
Intersection Capacity Utilization		77.6%	ICU Level of Service D
Analysis Period (min)		15	



Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations												
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	11	11	9	11	11	10	10	10	12	11	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	48		0	85		0	0		60	0		0
Storage Lanes	1		0	2		0	0		1	0		0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Leading Detector (ft)	48	50		50	50		50	50	50	50	50	
Trailing Detector (ft)	0	0		0	0		0	0	0	0	0	
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	1.00	0.95	0.95	0.97	0.95	0.95	0.95	0.95	1.00	0.95	0.95	0.95
Ped Bike Factor												
Frt		0.924			0.979				0.850		0.945	
Flt Protected	0.950			0.950				0.964			0.991	
Satd. Flow (prot)	1652	3161	0	3090	3349	0	0	3184	1409	0	3204	0
Flt Permitted	0.950			0.950				0.964			0.991	
Satd. Flow (perm)	1652	3161	0	3090	3349	0	0	3184	1409	0	3204	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		167			14				75		56	
Headway Factor	1.09	1.04	1.04	1.14	1.04	1.04	1.09	1.09	1.09	1.00	1.04	1.00
Link Speed (mph)		45			45			35			25	
Link Distance (ft)		1093			4064			1070			1297	
Travel Time (s)		16.6			61.6			20.8			35.4	
Volume (vph)	62	226	248	648	963	160	509	168	213	54	115	95
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.92	0.85	0.91	0.86	0.85	0.85	0.85	0.85	0.89	0.85	0.76	0.76
Growth Factor	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	7%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	72	284	292	806	1212	201	641	211	256	68	162	134
Lane Group Flow (vph)	72	576	0	806	1413	0	0	852	256	0	364	0
Turn Type	Prot			Prot			Split		pt+ov	Split		
Protected Phases	1	6		5	2		4	4	4 5	8	8	
Permitted Phases												
Detector Phases	1	6		5	2		4	4	4 5	8	8	
Minimum Initial (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Minimum Split (s)	8.5	20.5		8.5	20.5		20.5	20.5		20.0	20.0	
Total Split (s)	24.5	59.5	0.0	39.0	74.0	0.0	44.5	44.5	83.5	24.5	24.5	0.0
Total Split (%)	14.6%	35.5%	0.0%	23.3%	44.2%	0.0%	26.6%	26.6%	49.9%	14.6%	14.6%	0.0%
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	1.5	1.5		1.5	1.5		1.5	1.5		1.5	1.5	
Lead/Lag	Lead	Lag		Lead	Lag							
Lead-Lag Optimize?	Yes	Yes		Yes	Yes							
Recall Mode	None	Min		None	Min		None	None		None	None	
Act Effct Green (s)	12.2	47.2		34.5	69.6			40.0	74.6		18.7	
Actuated g/C Ratio	0.08	0.30		0.22	0.44			0.25	0.47		0.12	



Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
v/c Ratio	0.57	0.54		1.20	0.96			1.54dl	0.36		0.85	
Control Delay	76.0	33.9		153.9	57.8			104.0	12.3		72.4	
Queue Delay	0.0	0.0		0.0	0.0			0.0	0.0		0.0	
Total Delay	76.0	33.9		153.9	57.8			104.0	12.3		72.4	
LOS	E	C		F	E			F	B		E	
Approach Delay		38.6			92.7			82.8			72.4	
Approach LOS		D			F			F			E	

**Intersection Summary**

Area Type: Other  
 Cycle Length: 167.5  
 Actuated Cycle Length: 158.5  
 Natural Cycle: 130  
 Control Type: Actuated-Uncoordinated  
 Maximum v/c Ratio: 1.20  
 Intersection Signal Delay: 80.4      Intersection LOS: F  
 Intersection Capacity Utilization 91.1%      ICU Level of Service F  
 Analysis Period (min) 15  
 dl Defacto Left Lane. Recode with 1 though lane as a left lane.

Splits and Phases: 1: PR-2 & J Rosado Ave

ø1 24.5 s	ø2 74 s	ø4 44.5 s	ø8 24.5 s
ø5 39 s	ø6 59.5 s		



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)		0%	0%		0%	
Storage Length (ft)	220			400	0	0
Storage Lanes	1			1	2	0
Total Lost Time (s)	4.0	5.0	5.0	5.0	5.0	5.0
Leading Detector (ft)	50	50	50	50	50	
Trailing Detector (ft)	0	0	0	0	0	
Turning Speed (mph)	15			9	15	9
Lane Util. Factor	1.00	0.95	0.95	1.00	0.97	0.95
Ped Bike Factor						
Frt				0.850	0.906	
Flt Protected	0.950				0.982	
Satd. Flow (prot)	1770	3438	3539	1583	3215	0
Flt Permitted	0.950				0.982	
Satd. Flow (perm)	1770	3438	3539	1583	3215	0
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				817	397	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Link Speed (mph)		45	45		35	
Link Distance (ft)		4064	3282		1144	
Travel Time (s)		61.6	49.7		22.3	
Volume (vph)	144	357	1385	1046	228	386
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.85	0.88	0.83	0.75	0.94	0.95
Growth Factor	107%	107%	107%	107%	107%	107%
Heavy Vehicles (%)	2%	5%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Adj. Flow (vph)	181	434	1785	1492	260	435
Lane Group Flow (vph)	181	434	1785	1492	695	0
Turn Type	Prot			Free		
Protected Phases	5	2	6		4	
Permitted Phases				Free		
Detector Phases	5	2	6		4	
Minimum Initial (s)	4.0	4.0	4.0		4.0	
Minimum Split (s)	8.0	21.0	21.0		21.0	
Total Split (s)	24.0	64.0	40.0	0.0	55.0	0.0
Total Split (%)	20.2%	53.8%	33.6%	0.0%	46.2%	0.0%
Yellow Time (s)	2.0	3.0	3.0		3.0	
All-Red Time (s)	2.0	2.0	2.0		2.0	
Lead/Lag	Lead		Lag			
Lead-Lag Optimize?	Yes		Yes			
Recall Mode	None	Min	Min		None	
Act Effct Green (s)	12.9	52.6	35.6	76.5	13.7	
Actuated g/C Ratio	0.17	0.69	0.47	1.00	0.18	

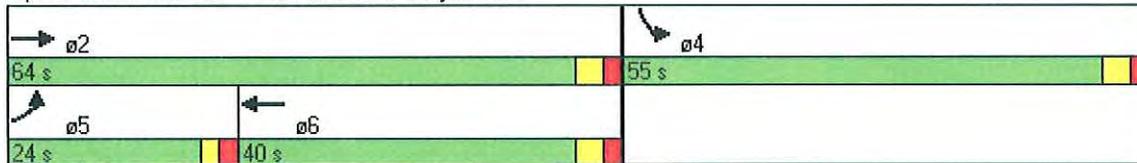


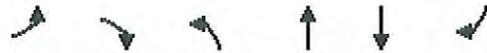
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
v/c Ratio	0.61	0.18	1.08	0.94	0.77	
Control Delay	33.6	5.1	72.0	14.7	13.2	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	33.6	5.1	72.0	14.7	13.2	
LOS	C	A	E	B	B	
Approach Delay		13.5	45.9		13.2	
Approach LOS		B	D		B	

**Intersection Summary**

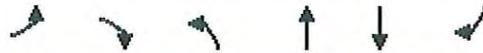
Area Type:	Other
Cycle Length:	119
Actuated Cycle Length:	76.5
Natural Cycle:	90
Control Type:	Actuated-Uncoordinated
Maximum v/c Ratio:	1.08
Intersection Signal Delay:	36.6
Intersection Capacity Utilization	81.2%
Analysis Period (min)	15
Intersection LOS:	D
ICU Level of Service	D

Splits and Phases: 2: PR-2 & V Rojas Ave





Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)	0%			0%	0%	
Storage Length (ft)	0	0	350			400
Storage Lanes	1	0	1			1
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Leading Detector (ft)	50		50	50	50	50
Trailing Detector (ft)	0		0	0	0	0
Turning Speed (mph)	15	9	15			9
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	1.00
Ped Bike Factor						
Frt	0.895					0.850
Flt Protected	0.989		0.950			
Satd. Flow (prot)	841	0	902	3539	3539	808
Flt Permitted	0.989		0.950			
Satd. Flow (perm)	841	0	902	3539	3539	808
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)	7					12
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Link Speed (mph)	15			45	45	
Link Distance (ft)	773			848	3282	
Travel Time (s)	35.1			12.8	49.7	
Volume (vph)	2	7	33	2551	574	11
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	1.00	1.00	1.00	0.79	0.94	1.00
Growth Factor	107%	107%	107%	107%	107%	107%
Heavy Vehicles (%)	100%	100%	100%	2%	2%	100%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Adj. Flow (vph)	2	7	35	3455	653	12
Lane Group Flow (vph)	9	0	35	3455	653	12
Turn Type			Prot			Free
Protected Phases	4		5	2	6	
Permitted Phases						Free
Detector Phases	4		5	2	6	
Minimum Initial (s)	10.0		4.0	4.0	4.0	
Minimum Split (s)	25.0		15.0	40.0	25.0	
Total Split (s)	25.0	0.0	20.0	45.0	25.0	0.0
Total Split (%)	35.7%	0.0%	28.6%	64.3%	35.7%	0.0%
Yellow Time (s)	3.0		3.0	3.0	3.0	
All-Red Time (s)	1.5		1.5	1.5	1.5	
Lead/Lag			Lead		Lag	
Lead-Lag Optimize?			Yes		Yes	
Recall Mode	None		None	Max	Max	
Act Effct Green (s)	11.5		10.4	86.9	76.9	92.3
Actuated g/C Ratio	0.11		0.11	0.94	0.83	1.00

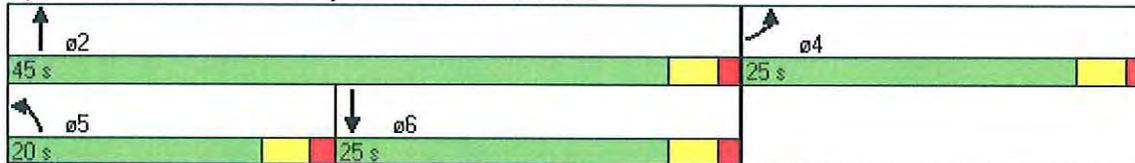


Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
v/c Ratio	0.09		0.37	1.04	0.22	0.01
Control Delay	14.6		23.0	33.3	4.3	0.0
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	14.6		23.0	33.3	4.3	0.0
LOS	B		C	C	A	A
Approach Delay	14.6			33.2	4.2	
Approach LOS	B			C	A	

**Intersection Summary**

Area Type:	Other
Cycle Length:	70
Actuated Cycle Length:	92.3
Natural Cycle:	150
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	1.04
Intersection Signal Delay:	28.5
Intersection Capacity Utilization	90.5%
Analysis Period (min)	15
Intersection LOS:	C
ICU Level of Service	E

Splits and Phases: 3: Project Access #1 & PR-2



							
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations				 	 		
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Volume (veh/h)	1	2	10	2582	572	9	
Peak Hour Factor	1.00	1.00	1.00	0.79	0.93	1.00	
Hourly flow rate (vph)	1	2	11	3497	658	10	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	Raised						
Median storage veh	0						
Upstream signal (ft)							
pX, platoon unblocked							
vC, conflicting volume	2428	329	668				
vC1, stage 1 conf vol	658						
vC2, stage 2 conf vol	1770						
vCu, unblocked vol	2428	329	668				
tC, single (s)	6.8	6.9	4.1				
tC, 2 stage (s)	5.8						
tF (s)	3.5	3.3	2.2				
p0 queue free %	98	100	99				
cM capacity (veh/h)	71	667	918				
Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3
Volume Total	3	11	1749	1749	329	329	10
Volume Left	1	11	0	0	0	0	0
Volume Right	2	0	0	0	0	0	10
cSH	175	918	1700	1700	1700	1700	1700
Volume to Capacity	0.02	0.01	1.03	1.03	0.19	0.19	0.01
Queue Length 95th (ft)	1	1	0	0	0	0	0
Control Delay (s)	25.9	9.0	0.0	0.0	0.0	0.0	0.0
Lane LOS	D	A					
Approach Delay (s)	25.9	0.0			0.0		
Approach LOS	D						
Intersection Summary							
Average Delay			0.0				
Intersection Capacity Utilization		86.4%		ICU Level of Service		E	
Analysis Period (min)		15					

Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations												
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	11	11	9	11	11	10	10	10	12	11	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	48		0	85		0	0		60	0		0
Storage Lanes	1		0	2		0	0		1	0		1
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Leading Detector (ft)	48	50		50	50		50	50	50	50	50	50
Trailing Detector (ft)	0	0		0	0		0	0	0	0	0	0
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	0.91	0.91	0.95	0.97	0.95	0.95	0.95	0.95	1.00	0.95	0.95	1.00
Ped Bike Factor												
Frnt		0.924			0.979				0.850			0.850
Flt Protected	0.950			0.950				0.964			0.985	
Satd. Flow (prot)	1503	3028	0	3090	3349	0	0	3184	1409	0	3370	1583
Flt Permitted	0.950			0.950				0.964			0.985	
Satd. Flow (perm)	1503	3028	0	3090	3349	0	0	3184	1409	0	3370	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		201			19				125			134
Headway Factor	1.09	1.04	1.04	1.14	1.04	1.04	1.09	1.09	1.09	1.00	1.04	1.00
Link Speed (mph)		45			45			25			25	
Link Distance (ft)		1093			4064			1070			1297	
Travel Time (s)		16.6			61.6			29.2			35.4	
Volume (vph)	62	226	248	648	963	160	509	168	213	54	115	95
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.92	0.85	0.91	0.86	0.85	0.85	0.85	0.85	0.89	0.85	0.76	0.76
Growth Factor	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	7%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	72	284	292	806	1212	201	641	211	256	68	162	134
Lane Group Flow (vph)	72	576	0	806	1413	0	0	852	256	0	230	134
Turn Type	Prot			Prot			Split		pt+ov	Split		Perm
Protected Phases	1	6		5	2		4	4	4 5	8	8	
Permitted Phases												8
Detector Phases	1	6		5	2		4	4	4 5	8	8	8
Minimum Initial (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	4.0
Minimum Split (s)	8.5	8.5		8.5	8.5		8.5	8.5		8.5	8.5	8.5
Total Split (s)	14.0	31.0	0.0	36.0	53.0	0.0	37.0	37.0	73.0	16.0	16.0	16.0
Total Split (%)	11.7%	25.8%	0.0%	30.0%	44.2%	0.0%	30.8%	30.8%	60.8%	13.3%	13.3%	13.3%
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	3.0
All-Red Time (s)	1.5	1.5		1.5	1.5		1.5	1.5		1.5	1.5	1.5
Lead/Lag	Lead	Lag		Lead	Lag							
Lead-Lag Optimize?	Yes	Yes		Yes	Yes							
Recall Mode	None	Min		None	Min		None	None		None	None	None
Act Effct Green (s)	8.8	23.4		31.6	48.7			32.6	64.2		11.0	11.0
Actuated g/C Ratio	0.07	0.20		0.27	0.42			0.28	0.55		0.09	0.09



Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
v/c Ratio	0.65	0.75		0.96	1.00			1.39dl	0.31		0.73	0.50
Control Delay	74.8	32.8		66.3	59.3			63.9	5.1		63.8	14.8
Queue Delay	0.0	0.0		0.0	0.0			0.0	0.0		0.0	0.0
Total Delay	74.8	32.8		66.3	59.3			63.9	5.1		63.8	14.8
LOS	E	C		E	E			E	A		E	B
Approach Delay		37.4			61.8			50.3			45.7	
Approach LOS		D			E			D			D	

**Intersection Summary**

Area Type: Other  
 Cycle Length: 120  
 Actuated Cycle Length: 116.7  
 Natural Cycle: 100  
 Control Type: Actuated-Uncoordinated  
 Maximum v/c Ratio: 1.00  
 Intersection Signal Delay: 53.9      Intersection LOS: D  
 Intersection Capacity Utilization 95.6%      ICU Level of Service F  
 Analysis Period (min) 15  
 dl Defacto Left Lane. Recode with 1 though lane as a left lane.

Splits and Phases: 1: PR-2 & J Rosado Ave

ø1	ø2	ø4	ø8
14 s	53 s	37 s	16 s
ø5	ø6		
36 s	31 s		



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↙	↕↕	↕↕	↗	↙↙	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)		0%	0%		0%	
Storage Length (ft)	220			400	0	0
Storage Lanes	1			1	2	0
Total Lost Time (s)	4.0	5.0	5.0	5.0	5.0	5.0
Leading Detector (ft)	50	50	50	50	50	
Trailing Detector (ft)	0	0	0	0	0	
Turning Speed (mph)	15			9	15	9
Lane Util. Factor	1.00	0.95	0.95	1.00	0.97	0.95
Ped Bike Factor						
Fr <sub>t</sub>				0.850	0.906	
Fl <sub>t</sub> Protected	0.950				0.982	
Satd. Flow (prot)	1770	3438	3539	1583	3215	0
Fl <sub>t</sub> Permitted	0.950				0.982	
Satd. Flow (perm)	1770	3438	3539	1583	3215	0
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				817	397	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Link Speed (mph)		45	45		35	
Link Distance (ft)		4064	3282		1144	
Travel Time (s)		61.6	49.7		22.3	
Volume (vph)	144	357	1385	1046	228	386
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.85	0.88	0.83	0.75	0.94	0.95
Growth Factor	107%	107%	107%	107%	107%	107%
Heavy Vehicles (%)	2%	5%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Adj. Flow (vph)	181	434	1785	1492	260	435
Lane Group Flow (vph)	181	434	1785	1492	695	0
Turn Type	Prot			Free		
Protected Phases	5	2	6		4	
Permitted Phases				Free		
Detector Phases	5	2	6		4	
Minimum Initial (s)	4.0	4.0	4.0		4.0	
Minimum Split (s)	8.0	21.0	21.0		21.0	
Total Split (s)	24.0	64.0	40.0	0.0	55.0	0.0
Total Split (%)	20.2%	53.8%	33.6%	0.0%	46.2%	0.0%
Yellow Time (s)	2.0	3.0	3.0		3.0	
All-Red Time (s)	2.0	2.0	2.0		2.0	
Lead/Lag	Lead		Lag			
Lead-Lag Optimize?	Yes		Yes			
Recall Mode	None	Min	Min		None	
Act Effct Green (s)	12.9	52.6	35.6	76.5	13.7	
Actuated g/C Ratio	0.17	0.69	0.47	1.00	0.18	

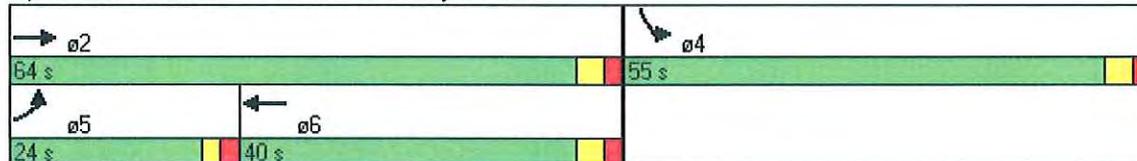


Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
v/c Ratio	0.61	0.18	1.08	0.94	0.77	
Control Delay	33.6	5.1	72.0	14.7	13.2	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	33.6	5.1	72.0	14.7	13.2	
LOS	C	A	E	B	B	
Approach Delay		13.5	45.9		13.2	
Approach LOS		B	D		B	

**Intersection Summary**

Area Type:	Other
Cycle Length:	119
Actuated Cycle Length:	76.5
Natural Cycle:	90
Control Type:	Actuated-Uncoordinated
Maximum v/c Ratio:	1.08
Intersection Signal Delay:	36.6
Intersection LOS:	D
Intersection Capacity Utilization	81.2%
ICU Level of Service	D
Analysis Period (min)	15

Splits and Phases: 2: PR-2 & V Rojas Ave





Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↘		↘	↑↑	↑↑	↘
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)	0%			0%	0%	
Storage Length (ft)	0	0	350			400
Storage Lanes	1	0	1			1
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Leading Detector (ft)	50		50	50	50	50
Trailing Detector (ft)	0		0	0	0	0
Turning Speed (mph)	15	9	15			9
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	1.00
Ped Bike Factor						
Fr <sub>t</sub>	0.895					0.850
Fl <sub>t</sub> Protected	0.989		0.950			
Satd. Flow (prot)	841	0	902	3539	3539	808
Fl <sub>t</sub> Permitted	0.989		0.950			
Satd. Flow (perm)	841	0	902	3539	3539	808
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)	7					12
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Link Speed (mph)	15			45	45	
Link Distance (ft)	773			848	3282	
Travel Time (s)	35.1			12.8	49.7	
Volume (vph)	2	7	33	2551	574	11
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	1.00	1.00	1.00	0.79	0.94	1.00
Growth Factor	107%	107%	107%	107%	107%	107%
Heavy Vehicles (%)	100%	100%	100%	2%	2%	100%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Adj. Flow (vph)	2	7	35	3455	653	12
Lane Group Flow (vph)	9	0	35	3455	653	12
Turn Type			Prot			Free
Protected Phases	4		5	2	6	
Permitted Phases						Free
Detector Phases	4		5	2	6	
Minimum Initial (s)	10.0		4.0	4.0	4.0	
Minimum Split (s)	25.0		15.0	40.0	25.0	
Total Split (s)	25.0	0.0	20.0	45.0	25.0	0.0
Total Split (%)	35.7%	0.0%	28.6%	64.3%	35.7%	0.0%
Yellow Time (s)	3.0		3.0	3.0	3.0	
All-Red Time (s)	1.5		1.5	1.5	1.5	
Lead/Lag			Lead		Lag	
Lead-Lag Optimize?			Yes		Yes	
Recall Mode	None		None	Max	Max	
Act Effct Green (s)	11.5		10.4	86.9	76.9	92.3
Actuated g/C Ratio	0.11		0.11	0.94	0.83	1.00

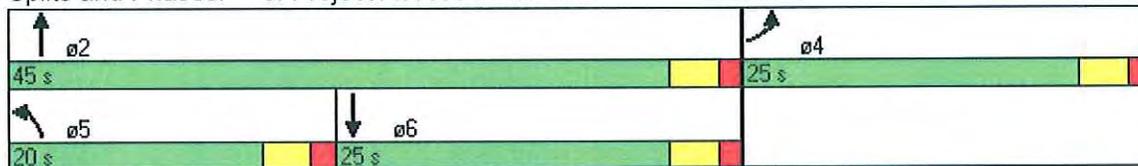


Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
v/c Ratio	0.09		0.37	1.04	0.22	0.01
Control Delay	14.6		23.0	33.3	4.3	0.0
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	14.6		23.0	33.3	4.3	0.0
LOS	B		C	C	A	A
Approach Delay	14.6			33.2	4.2	
Approach LOS	B			C	A	

**Intersection Summary**

Area Type:	Other
Cycle Length:	70
Actuated Cycle Length:	92.3
Natural Cycle:	150
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	1.04
Intersection Signal Delay:	28.5
Intersection Capacity Utilization	90.5%
Analysis Period (min)	15
Intersection LOS:	C
ICU Level of Service	E

Splits and Phases: 3: Project Access #1 & PR-2





Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Volume (veh/h)	1	2	10	2582	572	9
Peak Hour Factor	1.00	1.00	1.00	0.79	0.93	1.00
Hourly flow rate (vph)	1	2	11	3497	658	10
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	Raised					
Median storage veh)	0					
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	2428	329	668			
vC1, stage 1 conf vol	658					
vC2, stage 2 conf vol	1770					
vCu, unblocked vol	2428	329	668			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)	5.8					
tF (s)	3.5	3.3	2.2			
p0 queue free %	98	100	99			
cM capacity (veh/h)	71	667	918			

Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3
Volume Total	3	11	1749	1749	329	329	10
Volume Left	1	11	0	0	0	0	0
Volume Right	2	0	0	0	0	0	10
cSH	175	918	1700	1700	1700	1700	1700
Volume to Capacity	0.02	0.01	1.03	1.03	0.19	0.19	0.01
Queue Length 95th (ft)	1	1	0	0	0	0	0
Control Delay (s)	25.9	9.0	0.0	0.0	0.0	0.0	0.0
Lane LOS	D	A					
Approach Delay (s)	25.9	0.0			0.0		
Approach LOS	D						

Intersection Summary			
Average Delay		0.0	
Intersection Capacity Utilization	86.4%	ICU Level of Service	E
Analysis Period (min)	15		

Arecibo Resource Recovery Facility  
Existing PM Peak

Traffic Study  
3/25/2010

Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations												
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	11	11	9	11	11	12	11	12	12	11	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	48		0	85		0	0		0	0		0
Storage Lanes	1		0	2		0	0		0	0		0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Leading Detector (ft)	48	50		50	50		50	50		50	50	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	1.00	0.95	0.95	0.97	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor												
Frnt		0.922			0.979			0.937			0.950	
Flt Protected	0.950			0.950				0.979			0.987	
Satd. Flow (prot)	1652	3154	0	3090	3349	0	0	3138	0	0	3208	0
Flt Permitted	0.950			0.950				0.979			0.987	
Satd. Flow (perm)	1652	3154	0	3090	3349	0	0	3138	0	0	3208	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		202			13			117			46	
Headway Factor	1.09	1.04	1.04	1.14	1.04	1.04	1.00	1.04	1.00	1.00	1.04	1.00
Link Speed (mph)		35			45			35			25	
Link Distance (ft)		1093			4064			1070			1297	
Travel Time (s)		21.3			61.6			20.8			35.4	
Volume (vph)	98	366	388	368	552	91	262	86	310	86	125	103
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.91	0.93	0.90	0.96	0.95	0.95	0.76	0.77	0.93	0.93	0.84	0.86
Growth Factor	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	115	421	461	410	622	102	369	120	357	99	159	128
Lane Group Flow (vph)	115	882	0	410	724	0	0	846	0	0	386	0
Turn Type	Prot			Prot			Split			Split		
Protected Phases	1	6		5	2		4	4		8	8	
Permitted Phases												
Detector Phases	1	6		5	2		4	4		8	8	
Minimum Initial (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Minimum Split (s)	8.5	20.5		8.5	20.5		20.5	20.5		20.0	20.0	
Total Split (s)	24.5	59.5	0.0	24.5	59.5	0.0	44.5	44.5	0.0	24.5	24.5	0.0
Total Split (%)	16.0%	38.9%	0.0%	16.0%	38.9%	0.0%	29.1%	29.1%	0.0%	16.0%	16.0%	0.0%
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	1.5	1.5		1.5	1.5		1.5	1.5		1.5	1.5	
Lead/Lag	Lead	Lag		Lead	Lag							
Lead-Lag Optimize?	Yes	Yes		Yes	Yes							
Recall Mode	None	Min		None	Min		None	None		None	None	
Act Effct Green (s)	13.6	34.9		20.6	41.9			34.7			17.4	
Actuated g/C Ratio	0.11	0.28		0.16	0.33			0.27			0.14	

Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
v/c Ratio	0.65	0.87		0.81	0.65			0.89				0.80
Control Delay	63.2	35.9		67.1	38.5			45.3				55.8
Queue Delay	0.0	0.0		0.0	0.0			0.0				0.0
Total Delay	63.2	35.9		67.1	38.5			45.3				55.8
LOS	E	D		E	D			D				E
Approach Delay		39.1			48.9			45.3				55.8
Approach LOS		D			D			D				E

Intersection Summary

Area Type: Other  
 Cycle Length: 153  
 Actuated Cycle Length: 126.2  
 Natural Cycle: 90  
 Control Type: Actuated-Uncoordinated  
 Maximum v/c Ratio: 0.89  
 Intersection Signal Delay: 45.9  
 Intersection Capacity Utilization 81.7%  
 Analysis Period (min) 15

Intersection LOS: D  
 ICU Level of Service D

Splits and Phases: 1: PR-2 & J Rosado Ave

ø1	ø2	ø4	ø8
24.5 s	59.5 s	44.5 s	24.5 s
ø5	ø6		
24.5 s	59.5 s		

Arecibo Resource Recovery Facility  
Existing PM Peak

Traffic Study  
3/25/2010

Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)		0%	0%		0%	
Storage Length (ft)	220			0	0	0
Storage Lanes	1			0	2	0
Total Lost Time (s)	4.0	5.0	5.0	5.0	5.0	5.0
Leading Detector (ft)	50	50	50		50	
Trailing Detector (ft)	0	0	0		0	
Turning Speed (mph)	15			9	15	9
Lane Util. Factor	1.00	0.95	0.95	0.95	0.97	0.95
Ped Bike Factor						
Frt			0.935		0.942	
Flt Protected	0.950				0.970	
Satd. Flow (prot)	1770	3539	3309	0	3302	0
Flt Permitted	0.950				0.970	
Satd. Flow (perm)	1770	3539	3309	0	3302	0
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)			166		155	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Link Speed (mph)		45	45		35	
Link Distance (ft)		4064	4074		1144	
Travel Time (s)		61.6	61.7		22.3	
Volume (vph)	233	543	777	527	368	234
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.94	0.93	0.92	0.81	0.84	0.85
Growth Factor	107%	107%	107%	107%	107%	107%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Adj. Flow (vph)	265	625	904	696	469	295
Lane Group Flow (vph)	265	625	1600	0	764	0
Turn Type	Prot					
Protected Phases	5	2	6		4	
Permitted Phases						
Detector Phases	5	2	6		4	
Minimum Initial (s)	4.0	4.0	4.0		4.0	
Minimum Split (s)	8.0	21.0	21.0		21.0	
Total Split (s)	24.0	64.0	40.0	0.0	55.0	0.0
Total Split (%)	20.2%	53.8%	33.6%	0.0%	46.2%	0.0%
Yellow Time (s)	2.0	3.0	3.0		3.0	
All-Red Time (s)	2.0	2.0	2.0		2.0	
Lead/Lag	Lead		Lag			
Lead-Lag Optimize?	Yes		Yes			
Recall Mode	None	Min	Min		None	
Act Effct Green (s)	18.0	57.4	35.4		23.2	
Actuated g/C Ratio	0.20	0.63	0.39		0.26	



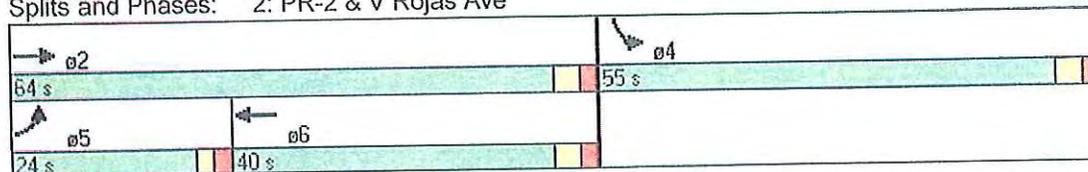
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
v/c Ratio	0.76	0.28	1.15		0.80	
Control Delay	46.0	8.6	101.9		26.0	
Queue Delay	0.0	0.0	0.0		0.0	
Total Delay	46.0	8.6	101.9		26.0	
LOS	D	A	F		C	
Approach Delay		19.7	101.9		26.0	
Approach LOS		B	F		C	

Intersection Summary

Area Type: Other  
 Cycle Length: 119  
 Actuated Cycle Length: 90.7  
 Natural Cycle: 80  
 Control Type: Actuated-Uncoordinated  
 Maximum v/c Ratio: 1.15  
 Intersection Signal Delay: 61.6  
 Intersection Capacity Utilization 85.7%  
 Analysis Period (min) 15

Intersection LOS: E  
 ICU Level of Service E

Splits and Phases: 2: PR-2 & V Rojas Ave





Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations												
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	11	11	9	11	11	12	11	12	12	11	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	48		0	85		0	0		0	0		0
Storage Lanes	1		0	2		0	0		0	0		0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Leading Detector (ft)	48	50		50	50		50	50		50	50	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	1.00	0.95	0.95	0.97	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Ped Bike Factor												
Frt		0.922			0.979			0.936			0.950	
Flt Protected	0.950			0.950				0.979			0.987	
Satd. Flow (prot)	1652	3154	0	3030	3349	0	0	3122	0	0	3208	0
Flt Permitted	0.950			0.950				0.979			0.987	
Satd. Flow (perm)	1652	3154	0	3030	3349	0	0	3122	0	0	3208	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		203			13			118			46	
Headway Factor	1.09	1.04	1.04	1.14	1.04	1.04	1.00	1.04	1.00	1.00	1.04	1.00
Link Speed (mph)		35			45			35			25	
Link Distance (ft)		1093			4064			1070			1297	
Travel Time (s)		21.3			61.6			20.8			35.4	
Volume (vph)	106	395	419	413	596	98	283	93	339	93	135	111
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.91	0.93	0.90	0.96	0.95	0.95	0.76	0.77	0.93	0.93	0.84	0.86
Growth Factor	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%
Heavy Vehicles (%)	2%	2%	2%	4%	2%	2%	2%	2%	3%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	125	454	498	460	671	110	398	129	390	107	172	138
Lane Group Flow (vph)	125	952	0	460	781	0	0	917	0	0	417	0
Turn Type	Prot			Prot			Split			Split		
Protected Phases	1	6		5	2		4	4		8	8	
Permitted Phases												
Detector Phases	1	6		5	2		4	4		8	8	
Minimum Initial (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Minimum Split (s)	8.5	20.5		8.5	20.5		20.5	20.5		20.0	20.0	
Total Split (s)	24.5	59.5	0.0	24.5	59.5	0.0	44.5	44.5	0.0	24.5	24.5	0.0
Total Split (%)	16.0%	38.9%	0.0%	16.0%	38.9%	0.0%	29.1%	29.1%	0.0%	16.0%	16.0%	0.0%
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	1.5	1.5		1.5	1.5		1.5	1.5		1.5	1.5	
Lead/Lag	Lead	Lag		Lead	Lag							
Lead-Lag Optimize?	Yes	Yes		Yes	Yes							
Recall Mode	None	Min		None	Min		None	None		None	None	
Act Effct Green (s)	15.0	41.1		20.1	46.2			40.2			19.1	
Actuated g/C Ratio	0.11	0.30		0.14	0.33			0.29			0.14	



Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
v/c Ratio	0.70	0.88		1.05	0.69			0.93			0.87	
Control Delay	69.9	40.1		111.9	42.2			57.8			67.6	
Queue Delay	0.0	0.0		0.0	0.0			0.0			0.0	
Total Delay	69.9	40.1		111.9	42.2			57.8			67.6	
LOS	E	D		F	D			E			E	
Approach Delay		43.5			68.1			57.8			67.6	
Approach LOS		D			E			E			E	

Intersection Summary

Area Type:	Other
Cycle Length:	153
Actuated Cycle Length:	138.7
Natural Cycle:	100
Control Type:	Actuated-Uncoordinated
Maximum v/c Ratio:	1.05
Intersection Signal Delay:	58.2
Intersection LOS:	E
Intersection Capacity Utilization	87.6%
ICU Level of Service	E
Analysis Period (min)	15

Splits and Phases: 1: PR-2 & J Rosado Ave

ø1 24.5 s	ø2 59.5 s	ø4 44.5 s	ø8 24.5 s
ø5 24.5 s	ø6 59.5 s		



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↔	↑↑	↑↑		↔↔	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)		0%	0%		0%	
Storage Length (ft)	220			0	0	0
Storage Lanes	1			0	2	0
Total Lost Time (s)	4.0	5.0	5.0	5.0	5.0	5.0
Leading Detector (ft)	50	50	50		50	
Trailing Detector (ft)	0	0	0		0	
Turning Speed (mph)	15			9	15	9
Lane Util. Factor	1.00	0.95	0.95	0.95	0.97	0.95
Ped Bike Factor						
Fr <sub>t</sub>			0.935		0.942	
Fl <sub>t</sub> Protected	0.950				0.970	
Satd. Flow (prot)	1770	3539	3291	0	3302	0
Fl <sub>t</sub> Permitted	0.950				0.970	
Satd. Flow (perm)	1770	3539	3291	0	3302	0
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)			163		155	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Link Speed (mph)		45	45		35	
Link Distance (ft)		4064	3282		1144	
Travel Time (s)		61.6	49.7		22.3	
Volume (vph)	252	590	855	569	397	253
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.94	0.93	0.92	0.81	0.84	0.85
Growth Factor	107%	107%	107%	107%	107%	107%
Heavy Vehicles (%)	2%	2%	3%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Adj. Flow (vph)	287	679	994	752	506	318
Lane Group Flow (vph)	287	679	1746	0	824	0
Turn Type	Prot					
Protected Phases	5	2	6		4	
Permitted Phases						
Detector Phases	5	2	6		4	
Minimum Initial (s)	4.0	4.0	4.0		4.0	
Minimum Split (s)	8.0	21.0	21.0		21.0	
Total Split (s)	24.0	64.0	40.0	0.0	55.0	0.0
Total Split (%)	20.2%	53.8%	33.6%	0.0%	46.2%	0.0%
Yellow Time (s)	2.0	3.0	3.0		3.0	
All-Red Time (s)	2.0	2.0	2.0		2.0	
Lead/Lag	Lead		Lag			
Lead-Lag Optimize?	Yes		Yes			
Recall Mode	None	Min	Min		None	
Act Effct Green (s)	19.5	58.8	35.2		25.7	
Actuated g/C Ratio	0.21	0.62	0.37		0.27	

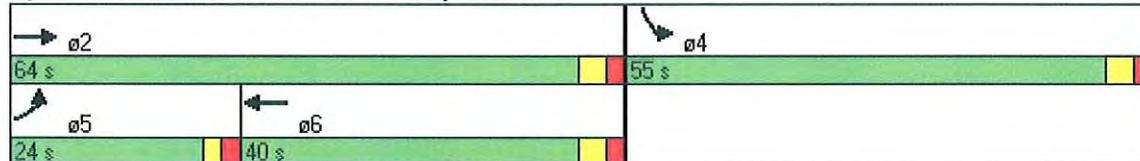


Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
v/c Ratio	0.79	0.31	1.31		0.82	
Control Delay	51.9	9.7	172.6		27.1	
Queue Delay	0.0	0.0	0.0		0.0	
Total Delay	51.9	9.7	172.6		27.1	
LOS	D	A	F		C	
Approach Delay		22.2	172.6		27.1	
Approach LOS		C	F		C	

**Intersection Summary**

Area Type:	Other
Cycle Length:	119
Actuated Cycle Length:	94.5
Natural Cycle:	90
Control Type:	Actuated-Uncoordinated
Maximum v/c Ratio:	1.31
Intersection Signal Delay:	97.6
Intersection Capacity Utilization	92.1%
Intersection LOS:	F
ICU Level of Service	F
Analysis Period (min)	15

Splits and Phases: 2: PR-2 & V Rojas Ave

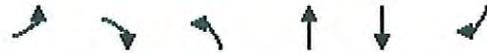




Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↘		↙	↑↑	↑↑	↘
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Volume (veh/h)	9	26	8	1487	991	2
Peak Hour Factor	1.00	1.00	1.00	0.87	0.91	1.00
Hourly flow rate (vph)	10	28	9	1829	1165	2
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None					
Median storage veh						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	2097	583	1167			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	2097	583	1167			
tC, single (s)	8.8	8.9	6.1			
tC, 2 stage (s)						
tF (s)	4.5	4.3	3.2			
p0 queue free %	26	90	97			
cM capacity (veh/h)	13	275	250			

Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3
Volume Total	37	9	914	914	583	583	2
Volume Left	10	9	0	0	0	0	0
Volume Right	28	0	0	0	0	0	2
cSH	44	250	1700	1700	1700	1700	1700
Volume to Capacity	0.84	0.03	0.54	0.54	0.34	0.34	0.00
Queue Length 95th (ft)	83	3	0	0	0	0	0
Control Delay (s)	229.6	19.9	0.0	0.0	0.0	0.0	0.0
Lane LOS	F	C					
Approach Delay (s)	229.6	0.1			0.0		
Approach LOS	F						

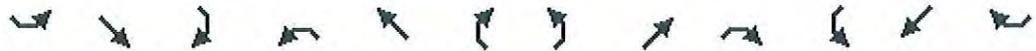
Intersection Summary			
Average Delay		2.9	
Intersection Capacity Utilization	54.0%		ICU Level of Service A
Analysis Period (min)		15	



Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↖		↖	↑↑	↑↑	↘
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Volume (veh/h)	7	8	2	1488	1015	2
Peak Hour Factor	1.00	1.00	1.00	0.87	0.91	1.00
Hourly flow rate (vph)	7	9	2	1830	1193	2
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	Raised					
Median storage (veh)	0					
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	2113	597	1196			
vC1, stage 1 conf vol	1193					
vC2, stage 2 conf vol	919					
vCu, unblocked vol	2113	597	1196			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)	5.8					
tF (s)	3.5	3.3	2.2			
p0 queue free %	93	98	100			
cM capacity (veh/h)	113	446	580			

Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3
Volume Total	16	2	915	915	597	597	2
Volume Left	7	2	0	0	0	0	0
Volume Right	9	0	0	0	0	0	2
cSH	188	580	1700	1700	1700	1700	1700
Volume to Capacity	0.09	0.00	0.54	0.54	0.35	0.35	0.00
Queue Length 95th (ft)	7	0	0	0	0	0	0
Control Delay (s)	25.9	11.2	0.0	0.0	0.0	0.0	0.0
Lane LOS	D	B					
Approach Delay (s)	25.9	0.0			0.0		
Approach LOS	D						

Intersection Summary			
Average Delay		0.1	
Intersection Capacity Utilization	54.0%	ICU Level of Service	A
Analysis Period (min)	15		



Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations	↘	↗		↘	↗			↗	↘		↗	↘
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	11	11	9	11	11	10	10	10	12	11	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	48		0	85		0	0		60	0		0
Storage Lanes	1		0	2		0	0		1	0		0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Leading Detector (ft)	48	50		50	50		50	50	50	50	50	
Trailing Detector (ft)	0	0		0	0		0	0	0	0	0	
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	1.00	0.95	0.95	0.97	0.95	0.95	0.95	0.95	1.00	0.95	0.95	0.95
Ped Bike Factor												
Frnt		0.922			0.979				0.850		0.950	
Flt Protected	0.950			0.950				0.964			0.987	
Satd. Flow (prot)	1652	3154	0	3030	3349	0	0	3184	1463	0	3208	0
Flt Permitted	0.950			0.950				0.964			0.987	
Satd. Flow (perm)	1652	3154	0	3030	3349	0	0	3184	1463	0	3208	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		177			13				185		42	
Headway Factor	1.09	1.04	1.04	1.14	1.04	1.04	1.09	1.09	1.09	1.00	1.04	1.00
Link Speed (mph)		45			45			35			25	
Link Distance (ft)		1093			4064			1070			1297	
Travel Time (s)		16.6			61.6			20.8			35.4	
Volume (vph)	106	395	419	413	596	98	283	93	339	93	135	111
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.91	0.93	0.90	0.96	0.95	0.95	0.76	0.77	0.93	0.93	0.84	0.86
Growth Factor	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%
Heavy Vehicles (%)	2%	2%	2%	4%	2%	2%	2%	2%	3%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	125	454	498	460	671	110	398	129	390	107	172	138
Lane Group Flow (vph)	125	952	0	460	781	0	0	527	390	0	417	0
Turn Type	Prot			Prot			Split		pt+ov	Split		
Protected Phases	1	6		5	2		4	4	4.5	8	8	
Permitted Phases												
Detector Phases	1	6		5	2		4	4	4.5	8	8	
Minimum Initial (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Minimum Split (s)	8.5	20.5		8.5	20.5		20.5	20.5		20.0	20.0	
Total Split (s)	24.5	59.5	0.0	39.0	74.0	0.0	44.5	44.5	83.5	24.5	24.5	0.0
Total Split (%)	14.6%	35.5%	0.0%	23.3%	44.2%	0.0%	26.6%	26.6%	49.9%	14.6%	14.6%	0.0%
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	1.5	1.5		1.5	1.5		1.5	1.5		1.5	1.5	
Lead/Lag	Lead	Lag		Lead	Lag							
Lead-Lag Optimize?	Yes	Yes		Yes	Yes							
Recall Mode	None	Min		None	Min		None	None		None	None	
Act Effct Green (s)	14.4	39.1		25.1	53.5			27.5	52.6		20.3	
Actuated g/C Ratio	0.11	0.30		0.19	0.41			0.21	0.40		0.15	



Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
v/c Ratio	0.70	0.90		0.79	0.57			1.15dl	0.56		0.79	
Control Delay	69.9	40.0		56.5	32.6			53.7	10.5		60.7	
Queue Delay	0.0	0.0		0.0	0.0			0.0	0.0		0.0	
Total Delay	69.9	40.0		56.5	32.6			53.7	10.5		60.7	
LOS	E	D		E	C			D	B		E	
Approach Delay		43.5			41.4			35.3			60.7	
Approach LOS		D			D			D			E	

**Intersection Summary**

Area Type: Other  
 Cycle Length: 167.5  
 Actuated Cycle Length: 131.2  
 Natural Cycle: 90  
 Control Type: Actuated-Uncoordinated  
 Maximum v/c Ratio: 0.90  
 Intersection Signal Delay: 42.7      Intersection LOS: D  
 Intersection Capacity Utilization 81.2%      ICU Level of Service D  
 Analysis Period (min) 15  
 dl Defacto Left Lane. Recode with 1 though lane as a left lane.

Splits and Phases: 1: PR-2 & J Rosado Ave

ø1 24.5 s	ø2 74 s	ø4 44.5 s	ø8 24.5 s
ø5 39 s	ø6 59.5 s		



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↘	↗↗	↗↗	↘	↘↘	↘↘
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)		0%	0%		0%	
Storage Length (ft)	220			400	0	0
Storage Lanes	1			1	2	0
Total Lost Time (s)	4.0	5.0	5.0	5.0	5.0	5.0
Leading Detector (ft)	50	50	50	50	50	
Trailing Detector (ft)	0	0	0	0	0	
Turning Speed (mph)	15			9	15	9
Lane Util. Factor	1.00	0.95	0.95	1.00	0.97	0.95
Ped Bike Factor						
Frt				0.850	0.942	
Flt Protected	0.950				0.970	
Satd. Flow (prot)	1770	3539	3505	1583	3302	0
Flt Permitted	0.950				0.970	
Satd. Flow (perm)	1770	3539	3505	1583	3302	0
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				739	155	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Link Speed (mph)		45	45		35	
Link Distance (ft)		4064	3282		1144	
Travel Time (s)		61.6	49.7		22.3	
Volume (vph)	252	590	855	569	397	253
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.94	0.93	0.92	0.81	0.84	0.85
Growth Factor	107%	107%	107%	107%	107%	107%
Heavy Vehicles (%)	2%	2%	3%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Adj. Flow (vph)	287	679	994	752	506	318
Lane Group Flow (vph)	287	679	994	752	824	0
Turn Type	Prot			Free		
Protected Phases	5	2	6		4	
Permitted Phases				Free		
Detector Phases	5	2	6		4	
Minimum Initial (s)	4.0	4.0	4.0		4.0	
Minimum Split (s)	8.0	21.0	21.0		21.0	
Total Split (s)	24.0	64.0	40.0	0.0	55.0	0.0
Total Split (%)	20.2%	53.8%	33.6%	0.0%	46.2%	0.0%
Yellow Time (s)	2.0	3.0	3.0		3.0	
All-Red Time (s)	2.0	2.0	2.0		2.0	
Lead/Lag	Lead		Lag			
Lead-Lag Optimize?	Yes		Yes			
Recall Mode	None	Min	Min		None	
Act Effct Green (s)	19.3	54.9	31.5	89.8	24.6	
Actuated g/C Ratio	0.21	0.61	0.35	1.00	0.27	

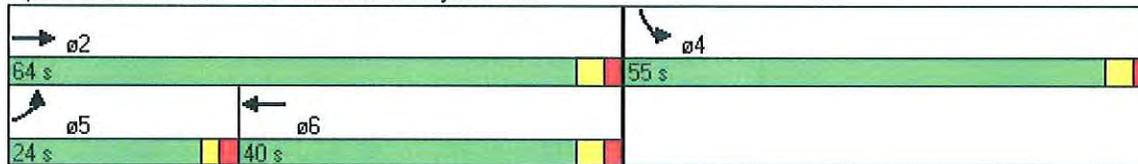


Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
v/c Ratio	0.76	0.31	0.81	0.48	0.81	
Control Delay	47.6	9.7	31.8	1.0	25.9	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	47.6	9.7	31.8	1.0	25.9	
LOS	D	A	C	A	C	
Approach Delay		21.0	18.5		25.9	
Approach LOS		C	B		C	

**Intersection Summary**

Area Type:	Other
Cycle Length:	119
Actuated Cycle Length:	89.8
Natural Cycle:	65
Control Type:	Actuated-Uncoordinated
Maximum v/c Ratio:	0.81
Intersection Signal Delay:	20.9
Intersection Capacity Utilization	72.5%
Analysis Period (min)	15
Intersection LOS:	C
ICU Level of Service	C

Splits and Phases: 2: PR-2 & V Rojas Ave





Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↘↘		↘	↑↑	↑↑	↘
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)	0%			0%	0%	
Storage Length (ft)	0	0	350			400
Storage Lanes	1	0	1			1
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Leading Detector (ft)	50		50	50	50	50
Trailing Detector (ft)	0		0	0	0	0
Turning Speed (mph)	15	9	15			9
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	1.00
Ped Bike Factor						
Frt	0.901					0.850
Flt Protected	0.987		0.950			
Satd. Flow (prot)	845	0	902	3539	3539	808
Flt Permitted	0.987		0.950			
Satd. Flow (perm)	845	0	902	3539	3539	808
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)	28					2
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Link Speed (mph)	15			45	45	
Link Distance (ft)	773			848	3282	
Travel Time (s)	35.1			12.8	49.7	
Volume (vph)	9	26	8	1487	991	2
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	1.00	1.00	1.00	0.87	0.91	1.00
Growth Factor	107%	107%	107%	107%	107%	107%
Heavy Vehicles (%)	100%	100%	100%	2%	2%	100%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Adj. Flow (vph)	10	28	9	1829	1165	2
Lane Group Flow (vph)	38	0	9	1829	1165	2
Turn Type			Prot			Free
Protected Phases	4		5	2	6	
Permitted Phases						Free
Detector Phases	4		5	2	6	
Minimum Initial (s)	10.0		4.0	4.0	4.0	
Minimum Split (s)	25.0		15.0	40.0	25.0	
Total Split (s)	25.0	0.0	20.0	45.0	25.0	0.0
Total Split (%)	35.7%	0.0%	28.6%	64.3%	35.7%	0.0%
Yellow Time (s)	3.0		3.0	3.0	3.0	
All-Red Time (s)	1.5		1.5	1.5	1.5	
Lead/Lag			Lead		Lag	
Lead-Lag Optimize?			Yes		Yes	
Recall Mode	None		None	Max	Max	
Act Effct Green (s)	11.7		7.7	83.8	79.9	94.1
Actuated g/C Ratio	0.11		0.08	0.89	0.85	1.00

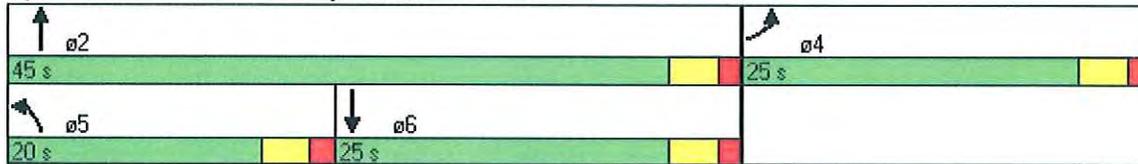


Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
v/c Ratio	0.31		0.13	0.58	0.39	0.00
Control Delay	13.4		25.4	3.8	4.7	0.0
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	13.4		25.4	3.8	4.7	0.0
LOS	B		C	A	A	A
Approach Delay	13.4			3.9	4.7	
Approach LOS	B			A	A	

**Intersection Summary**

Area Type:	Other
Cycle Length:	70
Actuated Cycle Length:	94.1
Natural Cycle:	70
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.58
Intersection Signal Delay:	4.3
Intersection Capacity Utilization	59.0%
Analysis Period (min)	15
Intersection LOS:	A
ICU Level of Service	B

Splits and Phases: 3: Project Access #1 & PR-2





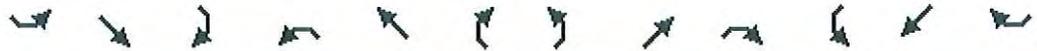
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Volume (veh/h)	7	8	2	1488	1015	2
Peak Hour Factor	1.00	1.00	1.00	0.87	0.91	1.00
Hourly flow rate (vph)	7	9	2	1830	1193	2
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	Raised					
Median storage veh	0					
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	2113	597	1196			
vC1, stage 1 conf vol	1193					
vC2, stage 2 conf vol	919					
vCu, unblocked vol	2113	597	1196			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)	5.8					
tF (s)	3.5	3.3	2.2			
p0 queue free %	93	98	100			
cM capacity (veh/h)	113	446	580			

Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3
Volume Total	16	2	915	915	597	597	2
Volume Left	7	2	0	0	0	0	0
Volume Right	9	0	0	0	0	0	2
cSH	188	580	1700	1700	1700	1700	1700
Volume to Capacity	0.09	0.00	0.54	0.54	0.35	0.35	0.00
Queue Length 95th (ft)	7	0	0	0	0	0	0
Control Delay (s)	25.9	11.2	0.0	0.0	0.0	0.0	0.0
Lane LOS	D	B					
Approach Delay (s)	25.9	0.0			0.0		
Approach LOS	D						

Intersection Summary			
Average Delay		0.1	
Intersection Capacity Utilization	54.0%		ICU Level of Service A
Analysis Period (min)		15	



Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations												
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	11	11	9	11	11	10	10	10	12	11	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	48		0	85		0	0		60	0		0
Storage Lanes	1		0	2		0	0		1	0		0
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Leading Detector (ft)	48	50		50	50		50	50	50	50	50	
Trailing Detector (ft)	0	0		0	0		0	0	0	0	0	
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	1.00	0.95	0.95	0.97	0.95	0.95	0.95	0.95	1.00	0.95	0.95	0.95
Ped Bike Factor												
Frt		0.922			0.979				0.850		0.950	
Flt Protected	0.950			0.950				0.964			0.987	
Satd. Flow (prot)	1652	3154	0	3030	3349	0	0	3184	1463	0	3208	0
Flt Permitted	0.950			0.950				0.964			0.987	
Satd. Flow (perm)	1652	3154	0	3030	3349	0	0	3184	1463	0	3208	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		176			14				182		42	
Headway Factor	1.09	1.04	1.04	1.14	1.04	1.04	1.09	1.09	1.09	1.00	1.04	1.00
Link Speed (mph)		45			45			35			25	
Link Distance (ft)		1093			4064			1070			1297	
Travel Time (s)		16.6			61.6			20.8			35.4	
Volume (vph)	120	447	474	467	674	111	320	105	383	105	153	126
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.92	0.94	0.91	0.98	0.96	0.96	0.77	0.77	0.95	0.94	0.85	0.87
Growth Factor	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%
Heavy Vehicles (%)	2%	2%	2%	4%	2%	2%	2%	2%	3%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	140	509	557	510	751	124	445	146	431	120	193	155
Lane Group Flow (vph)	140	1066	0	510	875	0	0	591	431	0	468	0
Turn Type	Prot			Prot			Split		pt+ov	Split		
Protected Phases	1	6		5	2		4	4	4 5	8	8	
Permitted Phases												
Detector Phases	1	6		5	2		4	4	4 5	8	8	
Minimum Initial (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Minimum Split (s)	8.5	20.5		8.5	20.5		20.5	20.5		20.0	20.0	
Total Split (s)	24.5	59.5	0.0	39.0	74.0	0.0	44.5	44.5	83.5	24.5	24.5	0.0
Total Split (%)	14.6%	35.5%	0.0%	23.3%	44.2%	0.0%	26.6%	26.6%	49.9%	14.6%	14.6%	0.0%
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	1.5	1.5		1.5	1.5		1.5	1.5		1.5	1.5	
Lead/Lag	Lead	Lag		Lead	Lag							
Lead-Lag Optimize?	Yes	Yes		Yes	Yes							
Recall Mode	None	Min		None	Min		None	None		None	None	
Act Effct Green (s)	16.4	48.1		29.6	61.3			33.0	62.6		20.6	
Actuated g/C Ratio	0.11	0.32		0.20	0.41			0.22	0.42		0.14	



Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
v/c Ratio	0.77	0.94		0.85	0.63			1.22dl	0.60		0.98	
Control Delay	81.5	49.9		66.7	37.2			62.4	13.4		95.3	
Queue Delay	0.0	0.0		0.0	0.0			0.0	0.0		0.0	
Total Delay	81.5	49.9		66.7	37.2			62.4	13.4		95.3	
LOS	F	D		E	D			E	B		F	
Approach Delay		53.6			48.0			41.8			95.3	
Approach LOS		D			D			D			F	

**Intersection Summary**

Area Type: Other  
 Cycle Length: 167.5  
 Actuated Cycle Length: 149.8  
 Natural Cycle: 90  
 Control Type: Actuated-Uncoordinated  
 Maximum v/c Ratio: 0.98  
 Intersection Signal Delay: 53.5                      Intersection LOS: D  
 Intersection Capacity Utilization 89.9%                      ICU Level of Service E  
 Analysis Period (min) 15

dl Defacto Left Lane. Recode with 1 though lane as a left lane.

Splits and Phases: 1: PR-2 & J Rosado Ave

ø1 24.5 s	ø2 74 s	ø4 44.5 s	ø8 24.5 s
ø5 39 s	ø6 59.5 s		



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↑	↑↑	↑↑	↑	↑↑↑	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)		0%	0%		0%	
Storage Length (ft)	220			400	0	0
Storage Lanes	1			1	2	0
Total Lost Time (s)	4.0	5.0	5.0	5.0	5.0	5.0
Leading Detector (ft)	50	50	50	50	50	
Trailing Detector (ft)	0	0	0	0	0	
Turning Speed (mph)	15			9	15	9
Lane Util. Factor	1.00	0.95	0.95	1.00	0.97	0.95
Ped Bike Factor						
Frt				0.850	0.942	
Flt Protected	0.950				0.970	
Satd. Flow (prot)	1770	3539	3505	1583	3302	0
Flt Permitted	0.950				0.970	
Satd. Flow (perm)	1770	3539	3505	1583	3302	0
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				738	156	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Link Speed (mph)		45	45		35	
Link Distance (ft)		4064	3282		1144	
Travel Time (s)		61.6	49.7		22.3	
Volume (vph)	284	667	966	643	449	286
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.95	0.94	0.93	0.82	0.85	0.86
Growth Factor	107%	107%	107%	107%	107%	107%
Heavy Vehicles (%)	2%	2%	3%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Adj. Flow (vph)	320	759	1111	839	565	356
Lane Group Flow (vph)	320	759	1111	839	921	0
Turn Type	Prot			Free		
Protected Phases	5	2	6		4	
Permitted Phases				Free		
Detector Phases	5	2	6		4	
Minimum Initial (s)	4.0	4.0	4.0		4.0	
Minimum Split (s)	8.0	21.0	21.0		21.0	
Total Split (s)	24.0	64.0	40.0	0.0	55.0	0.0
Total Split (%)	20.2%	53.8%	33.6%	0.0%	46.2%	0.0%
Yellow Time (s)	2.0	3.0	3.0		3.0	
All-Red Time (s)	2.0	2.0	2.0		2.0	
Lead/Lag	Lead		Lag			
Lead-Lag Optimize?	Yes		Yes			
Recall Mode	None	Min	Min		None	
Act Effct Green (s)	20.1	59.3	35.2	99.4	30.1	
Actuated g/C Ratio	0.20	0.60	0.35	1.00	0.30	

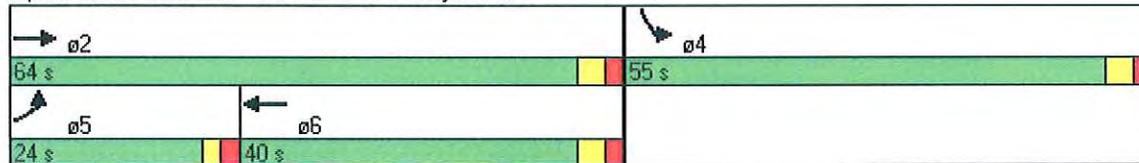


Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
v/c Ratio	0.89	0.36	0.90	0.53	0.83	
Control Delay	68.3	11.9	42.3	1.3	28.3	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	68.3	11.9	42.3	1.3	28.3	
LOS	E	B	D	A	C	
Approach Delay		28.6	24.7		28.3	
Approach LOS		C	C		C	

**Intersection Summary**

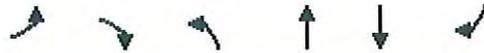
Area Type:	Other
Cycle Length:	119
Actuated Cycle Length:	99.4
Natural Cycle:	75
Control Type:	Actuated-Uncoordinated
Maximum v/c Ratio:	0.90
Intersection Signal Delay:	26.6
Intersection Capacity Utilization	80.4%
Analysis Period (min)	15
Intersection LOS:	C
ICU Level of Service	D

Splits and Phases: 2: PR-2 & V Rojas Ave





Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)	0%			0%	0%	
Storage Length (ft)	0	0	350			400
Storage Lanes	1	0	1			1
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Leading Detector (ft)	50		50	50	50	50
Trailing Detector (ft)	0		0	0	0	0
Turning Speed (mph)	15	9	15			9
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	1.00
Ped Bike Factor						
Frt	0.900					0.850
Flt Protected	0.987		0.950			
Satd. Flow (prot)	844	0	902	3539	3539	808
Flt Permitted	0.987		0.950			
Satd. Flow (perm)	844	0	902	3539	3539	808
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)	31					2
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Link Speed (mph)	15			45	45	
Link Distance (ft)	773			848	3282	
Travel Time (s)	35.1			12.8	49.7	
Volume (vph)	10	29	9	1680	1120	2
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	1.00	1.00	1.00	0.88	0.91	1.00
Growth Factor	107%	107%	107%	107%	107%	107%
Heavy Vehicles (%)	100%	100%	100%	2%	2%	100%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Adj. Flow (vph)	11	31	10	2043	1317	2
Lane Group Flow (vph)	42	0	10	2043	1317	2
Turn Type			Prot			Free
Protected Phases	4		5	2	6	
Permitted Phases						Free
Detector Phases	4		5	2	6	
Minimum Initial (s)	10.0		4.0	4.0	4.0	
Minimum Split (s)	25.0		15.0	40.0	25.0	
Total Split (s)	25.0	0.0	20.0	45.0	25.0	0.0
Total Split (%)	35.7%	0.0%	28.6%	64.3%	35.7%	0.0%
Yellow Time (s)	3.0		3.0	3.0	3.0	
All-Red Time (s)	1.5		1.5	1.5	1.5	
Lead/Lag			Lead		Lag	
Lead-Lag Optimize?			Yes		Yes	
Recall Mode	None		None	Max	Max	
Act Effct Green (s)	11.8		7.8	82.4	78.5	92.7
Actuated g/C Ratio	0.12		0.08	0.89	0.85	1.00

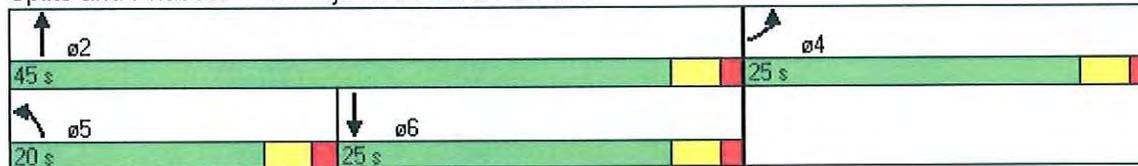


Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
v/c Ratio	0.34		0.14	0.65	0.44	0.00
Control Delay	13.4		25.7	5.0	5.2	0.0
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	13.4		25.7	5.0	5.2	0.0
LOS	B		C	A	A	A
Approach Delay	13.4			5.1	5.2	
Approach LOS	B			A	A	

**Intersection Summary**

Area Type:	Other
Cycle Length:	70
Actuated Cycle Length:	92.7
Natural Cycle:	75
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.65
Intersection Signal Delay:	5.2
Intersection Capacity Utilization	64.7%
Analysis Period (min)	15
Intersection LOS:	A
ICU Level of Service	C

Splits and Phases: 3: Project Access #1 & PR-2

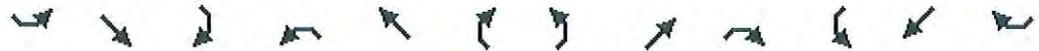




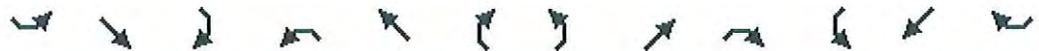
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↘↗		↘	↑↑	↑↑	↘
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Volume (veh/h)	8	9	2	1681	1147	2
Peak Hour Factor	1.00	1.00	1.00	0.88	0.91	1.00
Hourly flow rate (vph)	9	10	2	2044	1349	2
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	Raised					
Median storage (veh)	0					
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	2375	674	1351			
vC1, stage 1 conf vol	1349					
vC2, stage 2 conf vol	1026					
vCu, unblocked vol	2375	674	1351			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)	5.8					
tF (s)	3.5	3.3	2.2			
p0 queue free %	91	98	100			
cM capacity (veh/h)	93	397	505			

Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3
Volume Total	18	2	1022	1022	674	674	2
Volume Left	9	2	0	0	0	0	0
Volume Right	10	0	0	0	0	0	2
cSH	157	505	1700	1700	1700	1700	1700
Volume to Capacity	0.12	0.00	0.60	0.60	0.40	0.40	0.00
Queue Length 95th (ft)	10	0	0	0	0	0	0
Control Delay (s)	31.0	12.2	0.0	0.0	0.0	0.0	0.0
Lane LOS	D	B					
Approach Delay (s)	31.0	0.0			0.0		
Approach LOS	D						

Intersection Summary			
Average Delay		0.2	
Intersection Capacity Utilization	59.7%	ICU Level of Service	B
Analysis Period (min)	15		



Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations												
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	11	11	9	11	11	10	10	10	12	11	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	48		0	85		0	0		60	0		0
Storage Lanes	1		0	2		0	0		1	0		1
Total Lost Time (s)	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5
Leading Detector (ft)	48	50		50	50		50	50	50	50	50	50
Trailing Detector (ft)	0	0		0	0		0	0	0	0	0	0
Turning Speed (mph)	15		9	15		9	15		9	15		9
Lane Util. Factor	0.91	0.91	0.95	0.97	0.95	0.95	0.95	0.95	1.00	0.95	0.95	1.00
Ped Bike Factor												
Fr t		0.922			0.979				0.850			0.850
Fl t Protected	0.950			0.950				0.964			0.981	
Satd. Flow (prot)	1503	3022	0	3030	3349	0	0	3184	1463	0	3356	1583
Fl t Permitted	0.950			0.950				0.964			0.981	
Satd. Flow (perm)	1503	3022	0	3030	3349	0	0	3184	1463	0	3356	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		168			12				172			155
Headway Factor	1.09	1.04	1.04	1.14	1.04	1.04	1.09	1.09	1.09	1.00	1.04	1.00
Link Speed (mph)		45			45			25			25	
Link Distance (ft)		1093			4064			1070			1297	
Travel Time (s)		16.6			61.6			29.2			35.4	
Volume (vph)	120	447	474	467	674	111	320	105	383	105	153	126
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.92	0.94	0.91	0.98	0.96	0.96	0.77	0.77	0.95	0.94	0.85	0.87
Growth Factor	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%	107%
Heavy Vehicles (%)	2%	2%	2%	4%	2%	2%	2%	2%	3%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Adj. Flow (vph)	140	509	557	510	751	124	445	146	431	120	193	155
Lane Group Flow (vph)	140	1066	0	510	875	0	0	591	431	0	313	155
Turn Type	Prot			Prot			Split		pt+ov	Split		Perm
Protected Phases	1	6		5	2		4	4	4 5	8	8	
Permitted Phases												8
Detector Phases	1	6		5	2		4	4	4 5	8	8	8
Minimum Initial (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	4.0
Minimum Split (s)	8.5	8.5		8.5	8.5		8.5	8.5		8.5	8.5	8.5
Total Split (s)	29.0	63.5	0.0	39.0	73.5	0.0	51.0	51.0	90.0	23.5	23.5	23.5
Total Split (%)	16.4%	35.9%	0.0%	22.0%	41.5%	0.0%	28.8%	28.8%	50.8%	13.3%	13.3%	13.3%
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	3.0
All-Red Time (s)	1.5	1.5		1.5	1.5		1.5	1.5		1.5	1.5	1.5
Lead/Lag	Lead	Lag		Lead	Lag							
Lead-Lag Optimize?	Yes	Yes		Yes	Yes							
Recall Mode	None	Min		None	Min		None	None		None	None	None
Act Effct Green (s)	18.5	52.1		30.1	63.8			35.3	65.4		17.5	17.5
Actuated g/C Ratio	0.12	0.34		0.20	0.41			0.23	0.42		0.11	0.11

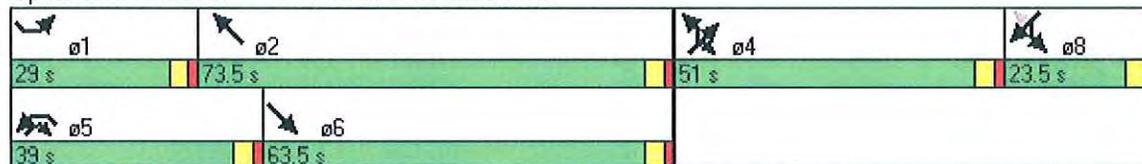


Lane Group	SEL	SET	SER	NWL	NWT	NWR	NEL	NET	NER	SWL	SWT	SWR
v/c Ratio	0.78	0.94		0.86	0.63			1.17dl	0.60		0.82	0.49
Control Delay	80.3	50.5		69.4	38.8			60.9	14.2		80.4	14.5
Queue Delay	0.0	0.0		0.0	0.0			0.0	0.0		0.0	0.0
Total Delay	80.3	50.5		69.4	38.8			60.9	14.2		80.4	14.5
LOS	F	D		E	D			E	B		F	B
Approach Delay		54.0			50.1			41.2			58.6	
Approach LOS		D			D			D			E	

**Intersection Summary**

Area Type: Other  
 Cycle Length: 177  
 Actuated Cycle Length: 153.9  
 Natural Cycle: 80  
 Control Type: Actuated-Uncoordinated  
 Maximum v/c Ratio: 0.94  
 Intersection Signal Delay: 50.0      Intersection LOS: D  
 Intersection Capacity Utilization 87.6%      ICU Level of Service E  
 Analysis Period (min) 15  
 dl Defacto Left Lane. Recode with 1 though lane as a left lane.

Splits and Phases: 1: PR-2 & J Rosado Ave





Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↔	↕↕	↕↕	↔	↕↕↕	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)		0%	0%		0%	
Storage Length (ft)	220			400	0	0
Storage Lanes	1			1	2	0
Total Lost Time (s)	4.0	5.0	5.0	5.0	5.0	5.0
Leading Detector (ft)	50	50	50	50	50	
Trailing Detector (ft)	0	0	0	0	0	
Turning Speed (mph)	15			9	15	9
Lane Util. Factor	1.00	0.95	0.95	1.00	0.97	0.95
Ped Bike Factor						
Frnt				0.850	0.942	
Flt Protected	0.950				0.970	
Satd. Flow (prot)	1770	3539	3505	1583	3302	0
Flt Permitted	0.950				0.970	
Satd. Flow (perm)	1770	3539	3505	1583	3302	0
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)				725	152	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Link Speed (mph)		45	45		35	
Link Distance (ft)		4064	3282		1144	
Travel Time (s)		61.6	49.7		22.3	
Volume (vph)	284	667	966	643	449	286
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.95	0.94	0.93	0.82	0.85	0.86
Growth Factor	107%	107%	107%	107%	107%	107%
Heavy Vehicles (%)	2%	2%	3%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Adj. Flow (vph)	320	759	1111	839	565	356
Lane Group Flow (vph)	320	759	1111	839	921	0
Turn Type	Prot			Free		
Protected Phases	5	2	6		4	
Permitted Phases				Free		
Detector Phases	5	2	6		4	
Minimum Initial (s)	4.0	4.0	4.0		4.0	
Minimum Split (s)	8.0	21.0	21.0		21.0	
Total Split (s)	26.0	64.0	40.0	0.0	55.0	0.0
Total Split (%)	21.5%	52.9%	33.1%	0.0%	45.5%	0.0%
Yellow Time (s)	2.0	3.0	3.0		3.0	
All-Red Time (s)	2.0	2.0	2.0		2.0	
Lead/Lag	Lead		Lag			
Lead-Lag Optimize?	Yes		Yes			
Recall Mode	None	Min	Min		None	
Act Effct Green (s)	22.1	61.3	35.2	102.2	30.9	
Actuated g/C Ratio	0.22	0.60	0.34	1.00	0.30	

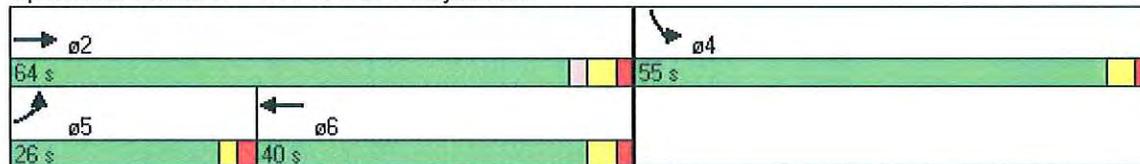


Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
v/c Ratio	0.84	0.36	0.92	0.53	0.83	
Control Delay	59.7	12.0	46.6	1.3	29.4	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	59.7	12.0	46.6	1.3	29.4	
LOS	E	B	D	A	C	
Approach Delay		26.1	27.1		29.4	
Approach LOS		C	C		C	

**Intersection Summary**

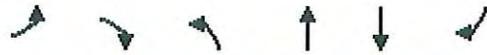
Area Type:	Other
Cycle Length:	121
Actuated Cycle Length:	102.2
Natural Cycle:	75
Control Type:	Actuated-Uncoordinated
Maximum v/c Ratio:	0.92
Intersection Signal Delay:	27.4
Intersection Capacity Utilization	80.4%
Analysis Period (min)	15
Intersection LOS:	C
ICU Level of Service	D

Splits and Phases: 2: PR-2 & V Rojas Ave





Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↘↘		↘	↑↑	↑↑	↘
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)	0%			0%	0%	
Storage Length (ft)	0	0	350			400
Storage Lanes	1	0	1			1
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Leading Detector (ft)	50		50	50	50	50
Trailing Detector (ft)	0		0	0	0	0
Turning Speed (mph)	15	9	15			9
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	1.00
Ped Bike Factor						
Frt	0.900					0.850
Flt Protected	0.987		0.950			
Satd. Flow (prot)	844	0	902	3539	3539	808
Flt Permitted	0.987		0.950			
Satd. Flow (perm)	844	0	902	3539	3539	808
Right Turn on Red		Yes				Yes
Satd. Flow (RTOR)	31					2
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Link Speed (mph)	15			45	45	
Link Distance (ft)	773			848	3282	
Travel Time (s)	35.1			12.8	49.7	
Volume (vph)	10	29	9	1680	1120	2
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	1.00	1.00	1.00	0.88	0.91	1.00
Growth Factor	107%	107%	107%	107%	107%	107%
Heavy Vehicles (%)	100%	100%	100%	2%	2%	100%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Adj. Flow (vph)	11	31	10	2043	1317	2
Lane Group Flow (vph)	42	0	10	2043	1317	2
Turn Type			Prot			Free
Protected Phases	4		5	2	6	
Permitted Phases						Free
Detector Phases	4		5	2	6	
Minimum Initial (s)	10.0		4.0	4.0	4.0	
Minimum Split (s)	25.0		15.0	40.0	25.0	
Total Split (s)	25.0	0.0	20.0	45.0	25.0	0.0
Total Split (%)	35.7%	0.0%	28.6%	64.3%	35.7%	0.0%
Yellow Time (s)	3.0		3.0	3.0	3.0	
All-Red Time (s)	1.5		1.5	1.5	1.5	
Lead/Lag			Lead		Lag	
Lead-Lag Optimize?			Yes		Yes	
Recall Mode	None		None	Max	Max	
Act Effct Green (s)	11.8		7.8	82.4	78.5	92.7
Actuated g/C Ratio	0.12		0.08	0.89	0.85	1.00

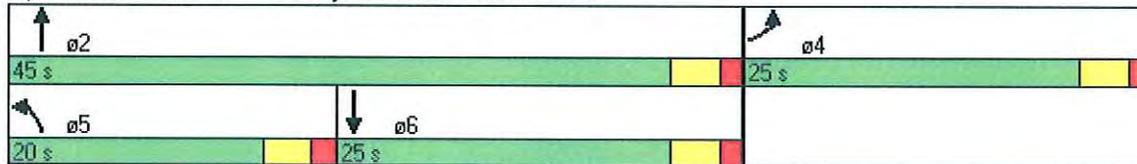


Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
v/c Ratio	0.34		0.14	0.65	0.44	0.00
Control Delay	13.4		25.7	5.0	5.2	0.0
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	13.4		25.7	5.0	5.2	0.0
LOS	B		C	A	A	A
Approach Delay	13.4			5.1	5.2	
Approach LOS	B			A	A	

**Intersection Summary**

Area Type:	Other
Cycle Length:	70
Actuated Cycle Length:	92.7
Natural Cycle:	75
Control Type:	Semi Act-Uncoord
Maximum v/c Ratio:	0.65
Intersection Signal Delay:	5.2
Intersection Capacity Utilization	64.7%
Analysis Period (min)	15
Intersection LOS:	A
ICU Level of Service	C

Splits and Phases: 3: Project Access #1 & PR-2





Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Volume (veh/h)	8	9	2	1681	1147	2
Peak Hour Factor	1.00	1.00	1.00	0.88	0.91	1.00
Hourly flow rate (vph)	9	10	2	2044	1349	2
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	Raised					
Median storage veh	0					
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	2375	674	1351			
vC1, stage 1 conf vol	1349					
vC2, stage 2 conf vol	1026					
vCu, unblocked vol	2375	674	1351			
tC, single (s)	6.8	6.9	4.1			
tC, 2 stage (s)	5.8					
tF (s)	3.5	3.3	2.2			
p0 queue free %	91	98	100			
cM capacity (veh/h)	93	397	505			

Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3
Volume Total	18	2	1022	1022	674	674	2
Volume Left	9	2	0	0	0	0	0
Volume Right	10	0	0	0	0	0	2
cSH	157	505	1700	1700	1700	1700	1700
Volume to Capacity	0.12	0.00	0.60	0.60	0.40	0.40	0.00
Queue Length 95th (ft)	10	0	0	0	0	0	0
Control Delay (s)	31.0	12.2	0.0	0.0	0.0	0.0	0.0
Lane LOS	D	B					
Approach Delay (s)	31.0	0.0			0.0		
Approach LOS	D						

Intersection Summary			
Average Delay		0.2	
Intersection Capacity Utilization	59.7%	ICU Level of Service	B
Analysis Period (min)	15		



# APPENDIX – I

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## Maximum Daily Heavy Vehicle Volume Distribution

Type of Heavy Vehicle	Maximum Daily Volume	Comments
Fuel Receiving: Transfer Trailers	65	Trailer capacity is approximately 100 yd <sup>3</sup> . Trailer length is approximately 50 ft.
Fuel Receiving: Roll-Off and Collection Trucks	189	Collection trucks: front end loaders and rear loaders.
Bottom Ash Metals Recovery	6	Roll-off truck.
Boiler Aggregate	14	Trailer capacity is between 40-60 yd <sup>3</sup> .
Conditioned Fly Ash	16	Trailer capacity is between 40-60 yd <sup>3</sup> .
Lime Delivery Truck	1	Pneumatic tankers (800-100 ft <sup>3</sup> ).
Alternative Fuels	20	Fuel tanker.
<b>Maximum Total Daily Heavy Vehicle Volume</b>	<b>311</b>	

Energy Answers International  
 Arecibo Puerto Rico  
 70MW Resource Recovery Facility  
 RRF

Note: 5 shift crews needed to cover Power Block. 3 shift crews needed to cover Processing. Two (main) shifts needed to cover ash processing.

**70MW - Puerto Rico Facility**

Position	Status	EX/NE	Hourly Rate	# of Positions	Shift Worked			
					1st	2nd	3rd	4th
<b>Administration</b>								
Facility Manager / General Manager	FT	EX		1	1			
O&M Manager	FT	EX		1	1			
Maintenance Manager	FT	EX		1	1			
Plant Engineer	FT	EX		1	1			
Environmental Health/Safety Manager	FT	EX		1	1			
HR Specialist	FT	EX		1	1			
Office Manager	FT	EX		1	1			
Plant Accountant	FT	EX		1	1			
Receptionist	FT	NE		1	1			
A/R Clerk	FT	NE		1	1			
A/P Clerk	FT	NE		1	1			
<b>Waste Processing Operations</b>								
Shift Supervisor - Fuels/Ash	FT	EX		3	1	1	1	
Heavy Equip. Opr. - MSW to PRF	FT	NE		6	2	2	2	
Utility Operator - Picker (front end)	FT	NE		6	2	2	2	
CRO - Fuel Handling	FT	NE		3	1	1	1	
Plant Operator - Fuels	FT	NE		12	4	4	4	
Weighmaster	FT	NE		2	1	1		
Utility Operator	FT	NE		4	2	2		
<b>Power Block Operations</b>								
Shift Supervisor - Power	FT	EX		5	2	1	1	1
CRO - Power	FT	NE		5	2	1	1	1
Assistant CRO - Power	FT	NE		5	2	1	1	1
Plant Operator - Power	FT	NE		10	4	2	2	2
Utility Operator-Power Block	FT	NE		5	2	1	1	1
Water Treatment Technician	FT	NE		1	1			
<b>Ash Processing</b>								
Plant Operator - Ash	FT	NE		4	2	2		
Utility Operator - Picker (ash)	FT	NE		2	1	1		
Heavy Equipment Operator-Ash	FT	NE		6	2	2	1	1
<b>Maintenance</b>								
Sr. Maintenance Mechanic	FT	NE		9	4	1	3	1
Jr. Maintenance Mechanic	FT	NE		8	3	1	3	1
Sr. Electrician	FT	NE		2	2			
Electrician	FT	NE		2	2			
I & C Technician	FT	NE		2	2			
Utility Worker	FT	NE		14	2	2	8	2
Warehouse Attendant	FT	NE		1	1			
<b>Total Plant Employees</b>				<b>128</b>				
<b>Tractor Trailer Drivers</b>				<b>13</b>				
<b>Corporate Management</b>				<b>9</b>				
<b>Total Employment</b>				<b>150</b>				



# APPENDIX – J

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PM Peak Hour Analysis

Existing (2010) vs. Opening Day (2013) Conditions

PR-2, PR-10, & Juan Rosado Ave.

Movement	2010			2013 with Improvements		
	Delay (sec)	LOS	Queue (veh)	2013 Delays with existing traffic signal times and geometry	2013 Allowed Delay (sec)	Queue (veh)
EB-L	63.2	E	7	69.9	< 80	9
EB-T	35.9	D	16	40.1	50.9	21
EB-R						
WB-L	67.1	E	13	111.9	< 80	13
WB-T	38.5	D	15	42.2	53.5	17
WB-R						
NB-L						
NB-T	45.3	D	15	57.8	60.3	12
NB-R						
SB-L						
SB-T	55.8	E	9	67.6	< 80	13
SB-R						

Existing (2010) vs. Opening Day (2018) Conditions

PR-2, PR-10, & Juan Rosado Ave.

Movement	2010			2018 with Improvements		
	Delay (sec)	LOS	Queue (veh)	2018 Delays with 2013 Conditions	2018 Allowed delay (sec)	Queue (veh)
EB-L	63.2	E	7	81.5	< 80	11
EB-T	35.9	D	16	49.9	50.9	30
EB-R						
WB-L	67.1	E	13	66.7	< 80	16
WB-T	38.5	D	15	37.2	53.5	21
WB-R						
NB-L						
NB-T	45.3	D	15	62.4	60.3	14
NB-R						
SB-L						
SB-T	55.8	E	9	95.3	< 80	10
SB-R						

PR-2 & Victor Rojas Ave.

Movement	2010			2013 with Improvements		
	Delay (sec)	LOS	Queue (veh)	2013 Delays with existing traffic signal times and geometry	2013 Allowed Delay (sec)	Queue (veh)
EB-L	46	D	12	51.9	61	14
EB-T	8.6	A	6	9.7	28.6	7
WB-T	101.9	F	33	172.6	< 80	17
WB-R						
SB-L	26	C	9	27.1	41	10
SB-R						

PR-2 & Victor Rojas Ave.

Movement	2010			2018 with Improvements		
	Delay (sec)	LOS	Queue (veh)	2018 Delays with 2013 Conditions	2018 Allowed delay (sec)	Queue (veh)
EB-L	46	D	12	68.3	61	17
EB-T	8.6	A	6	11.9	28.6	9
WB-T	101.9	F	33	42.3	< 80	24
WB-R						
SB-L	26	C	9	28.3	41	12
SB-R						

PR-2 & Access #1

Movement	2010			2013 with Improvements		
	Delay (sec)	LOS	Queue (veh)	2013 Delays with existing geometry	2013 Allowed Delay (sec)	Queue (veh)
NB-L	N/A	N/A	N/A	19.9	N/A	1
NB-T	N/A	N/A	N/A	N/A	N/A	10
SB-T	N/A	N/A	N/A	N/A	N/A	9
SB-R	N/A	N/A	N/A	229.6	N/A	1
EB-L						
EB-R						

PR-2 & Access #1

Movement	2010			2018 with Improvements		
	Delay (sec)	LOS	Queue (veh)	2018 Delays with 2013 Conditions	2018 Allowed delay (sec)	Queue (veh)
NB-L	N/A	N/A	N/A	25.7	N/A	1
NB-T	N/A	N/A	N/A	5	N/A	13
SB-T	N/A	N/A	N/A	5.2	N/A	11
SB-R	N/A	N/A	N/A	13.4	N/A	1
EB-L						
EB-R						

PR-2 & Access #2

Movement	2010			2013 with Improvements		
	Delay (sec)	LOS	Queue (veh)	2013 Delays with existing geometry	2013 Allowed Delay (sec)	Queue (veh)
NB-L	N/A	N/A	N/A	11.2	N/A	0
NB-T						
SB-T						
SB-R	N/A	N/A	N/A	25.9	N/A	1
EB-L						
EB-R						

PR-2 & Access #2

Movement	2010			2018 with Improvements		
	Delay (sec)	LOS	Queue (veh)	2018 Delays with 2013 Conditions	2018 Allowed delay (sec)	Queue (veh)
NB-L	N/A	N/A	N/A	12.2	N/A	0
NB-T						
SB-T						
SB-R	N/A	N/A	N/A	31	N/A	1
EB-L						
EB-R						



# APPENDIX – K

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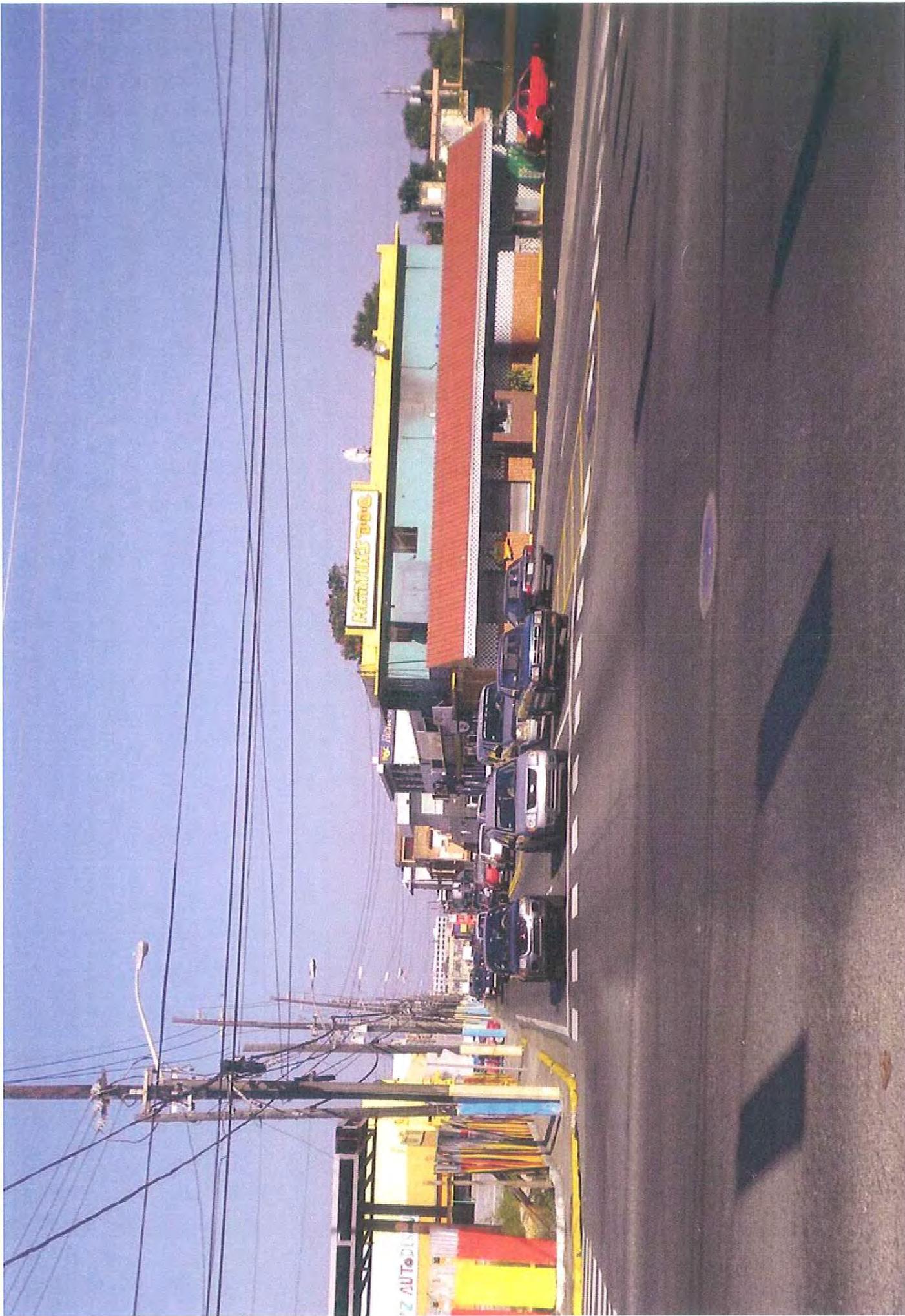


# **Intersection #1**

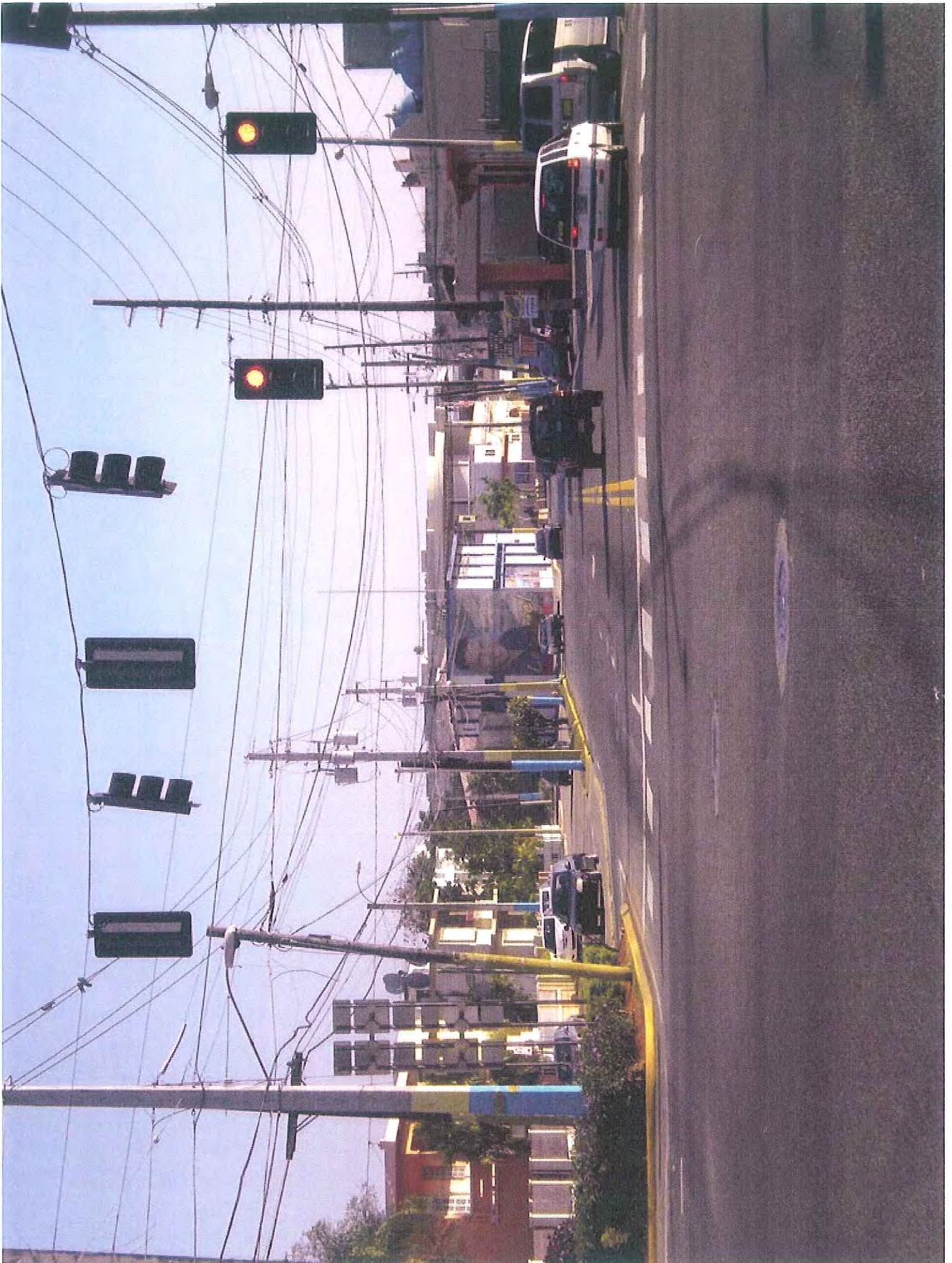
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**PR-2, PR-10 & Juan Rosado Avenue**











## **Intersection #2**

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**PR-2 & Victor Rojas Avenue**











**PR-2 Km. 73.1**

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**Proposed Project Area**



